COMMENTS: _____

PERMIT/APPLICATION NO _____
ZONING DISTRICT _____
MAGISTERIAL DISTRICT _____
DATE SUBMITTED _____

County of Prince Edward

PLEASE PRINT OR TYPE

PRINCE EDWARD COUNTY APPLICATION FOR SPECIAL USE PERMIT

TO: PRINCE EDWARD COUNTY PLANNING COMMISSION SPECIAL EXCEPTION REQUESTED:
VIA: ZONING ADMINISTRATOR

The undersigned owner of the following described property hereby applies for a Special Use permit as provided in Section 5-124 of Article V, Site Plan requirements are found in Section 4-100 of Article IV Development Standards of the Zoning Ordinance of Prince Edward County, Virginia.

Applicant's Name: Jonathan Yates for Milestone Tower Limited Partnership-IV (d/b/a Milestone Towers)
Applicant's Address: 105 Broad Street, 3rd Floor, Charleston, SC 29401
Applicant's Telephone Number: (843) 414-9754

Present Land Use: Agriculture Vacant

Legal Description of Property with Deed Book and Page No. or Instrument No.
Please see Exhibit "12" and "13" in the attached narrative.

Tax Map # 046-A-50 Acreage : 56.25

Narrative statement evaluating effects on adjoining properties (noise, odor, dust, fumes, etc.): (Attach additional sheet if necessary.) Please see attached project narrative.

Statement of general compatibility with adjacent and other properties in the zoning district. (Attach additional sheet if necessary.) Please see attached project narrative.

Height of Principal Building (s): Feet 195-feet **(revised to 255')** Stories N/A

APPLICANT'S STATEMENT: (if not owner(s) of property):

I hereby certify that I have the authority to make the foregoing application, that the information given is complete and correct to the best of my knowledge, and that development and/or construction will conform with the regulations as set forth in the Prince Edward County Zoning Ordinance as written and also with the description contained in this permit application.

Jonathan L Yates 02/11/2026
Signature of Applicant (if not property owner) Date

PROPERTY OWNER(S) STATEMENT:

I hereby certify that I/We own the above described property, that the information given is complete and correct to the best of my knowledge, and the above person(s), group, corporation, or agent has the full and complete permission of the undersigned owner(s) to make application for a Conditional Use permit as set forth in the Prince Edward County Zoning Ordinance as written.

Robert Alton Davis 8-17-23
Signature of Property Owner(s) Date

Signature of Property Owner(s) Date

Signature of Property Owner(s) Date

NOTE: THIS PERMIT APPLICATION IS NOT VALID UNLESS ALL PROPERTY OWNER(S) SIGNATURES ARE AFFIXED AND DATED. ATTACH ADDITIONAL SHEETS IF NECESSARY.

Application Fee $300.00 Fee Received by RMLowe Date 8/30/2023

The above mentioned application charges are nonrefundable, regardless of whether the permit application is approved or denied once submitted.

All checks for payment should be made payable to: Treasurer, Prince Edward County, Virginia.

Mail to: Department of Planning & Community Development
P. O. Box 382
Farmville, VA 23901
(434) 392-8837

COMMENTS: _____

PERMIT/APPLICATION NO _____
ZONING DISTRICT _____
MAGISTERIAL DISTRICT _____
DATE SUBMITTED _____

County of Prince Edward

PLEASE PRINT OR TYPE

PRINCE EDWARD COUNTY APPLICATION FOR SPECIAL USE PERMIT

TO: PRINCE EDWARD COUNTY PLANNING COMMISSION SPECIAL EXCEPTION REQUESTED:
VIA: ZONING ADMINISTRATOR

The undersigned owner of the following described property hereby applies for a Special Use permit as provided in Section 5-124 of Article V, Site Plan requirements are found in Section 4-100 of Article IV Development Standards of the Zoning Ordinance of Prince Edward County, Virginia.

Applicant's Name: Jonathan Yates for Milestone Tower Limited Partnership-IV (d/b/a Milestone Towers)
Applicant's Address: 105 Broad Street, 3rd Floor, Charleston, SC 29401
Applicant's Telephone Number: (843) 414-0754

Present Land Use: Agriculture Vacant

Legal Description of Property with Deed Book and Page No. or Instrument No. _____
Please see Exhibit "12" and "13" in the attached narrative.

Tax Map # 046-A-50 Acreage : 56.25

Narrative statement evaluating effects on adjoining properties (noise, odor, dust, fumes, etc.): (Attach additional sheet if necessary.) Please see attached project narrative.

Statement of general compatibility with adjacent and other properties in the zoning district. (Attach additional sheet if necessary.) Please see attached project narrative.

Height of Principal Building (s): Feet 195-feet **(revised to 255')** Stories N/A

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Jonathan L Yates 02/11/2026
Signature of Applicant (if not property owner) Date

PROPERTY OWNER(S) STATEMENT:

I hereby certify that I/We own the above described property, that the information given is complete and correct to the best of my knowledge, and the above person(s), group, corporation, or agent has the full and complete permission of the undersigned owner(s) to make application for a Conditional Use permit as set forth in the Prince Edward County Zoning Ordinance as written.

Robert Thomas II 8-17-23
Signature of Property Owner(s) Date

_____ _____
Signature of Property Owner(s) Date

_____ _____
Signature of Property Owner(s) Date

NOTE: THIS PERMIT APPLICATION IS NOT VALID UNLESS ALL PROPERTY OWNER(S) SIGNATURES ARE AFFIXED AND DATED. ATTACH ADDITIONAL SHEETS IF NECESSARY.

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Mail to: Department of Planning &
Community Development
P. O. Box 382
Farmville, VA 23901
(434) 392-8837

HELLMAN YATES

JONATHAN L. YATES
DIRECT VOICE 843 414-9754
JLY@HELLMANYATES.COM

HELLMAN & YATES, PA
145 KING STREET, SUITE 102
CHARLESTON, SOUTH CAROLINA 29401
V 843 266-9099
F 843 266-9188

February 11, 2026

VIA FEDERAL EXPRESS

Robert Love
Director of Planning and Community Development
111 N. South Street, 3rd Floor
Farmville, VA 23901
(434) 414-3037

Re: Application for construction of a 255-foot self-support style wireless telecommunications facility located at 4451 Five Forks Road Pamplin, VA 23958 (Parcel ID # 046-A-50) on behalf of Milestone and Verizon Wireless.

Dear Mr. Love,

Enclosed, please find the revised application of Milestone Towers and Verizon Wireless for a proposed 255-foot self-support style wireless telecommunications facility with a 4 ft. lightning rod for a total height of 259 ft. The original application proposed a 199' monopole-style facility, which was then put on hold pending the ordinance amendment and in order to redesign the facility to accommodate Prince Edward County emergency communications equipment. All other details of the original application remain the same. The proposed facility will be on the property of Robert Alton Gaunce, which is located at 4451 Five Forks Road, Pamplin, VA and is designated as Prince Edward County Parcel ID 046-A-50. This is a very important facility to allow Verizon Wireless to improve its coverage for both voice and advanced data in this section of Prince Edward County. The facility will also host Prince Edward County's emergency communications equipment with the revised height of 255 feet. The Site Plan and Drawings have been approved by John Harman of Motorola Solutions, Prince Edward County's consultant for emergency communications.

We have taken the liberty of recasting the relevant sections of the Prince Edward County Zoning Ordinance, with our answer to the relevant section in bold beneath. As will be evident from a review of the attached, Milestone, Verizon Wireless, and Prince Edward County have not only met, but have exceeded, all of the necessary requirements for approval under the Prince Edward County Zoning Ordinance.

Towers

(A) *Intent:* These minimum standards are intended to govern the location of all towers and the installation of antennas and accessory equipment structures.

(B) Towers, with related unmanned equipment buildings, shall be permitted only by special use permit in zoning districts as specified in article II, District Regulations. Applicants are encouraged to consider properties owned by the county when locating towers.

Applicant accepts and acknowledges this provision. Please see the Site Plan and Drawings by Virginia Professional Engineer Stuart P. Patterson attached hereto as Exhibit "1" and incorporated herein by reference. The Gaunce property is located in the A-1 zoning district.

(C) *General standards:*

1. No tower or related facilities shall be located within 500 feet of any residential district.

Applicant accepts and acknowledges this provision. The proposed facility is not located within 500 ft. of any residential district, as shown on sheet Z-1 of the Site Plan and Drawings, in Exhibit "1".

2. No new tower shall be permitted unless the applicant demonstrates to the reasonable satisfaction of the board of supervisors that no existing tower or structure can accommodate the proposed antenna. Evidence submitted to demonstrate that no existing tower or structure can accommodate the applicant's proposed antenna may consist of any of the following:
   a. No existing towers or structures are located within the geographic area required to meet the applicant's engineering requirements.
   b. Existing towers or structures are not of sufficient height to meet the applicant's engineering requirements.
   c. Existing towers or structures are not of sufficient structural strength to support the applicant's proposed antenna or related equipment.
   d. The applicant's proposed antenna would cause electromagnetic interference with existing antenna, or the antenna on the existing towers, or structures would cause interference with the applicants proposed antenna.
   e. The applicant demonstrates that there are other limiting factors that render existing towers or structures unsuitable.

Applicant accepts and acknowledges this provision. Please see the Alternative Candidate Analysis by Matt Penning of Milestone Towers attached hereto as Exhibit "2" and incorporated herein by reference. The closest existing tower is 2.334 miles to the northwest, and it will not allow Verizon Wireless to provide coverage to the intended area.

3. No tower shall exceed 199 feet in height, including antennas.

   The original application proposed a 199 ft. wireless telecommunications facility, as stated in this requirement. After the application was put on hold for the passage of the amended ordinance, the revised application proposes a 255 ft. wireless telecommunications facility with a 4 ft. lightning rod for a total height of 259 ft. to accommodate Prince Edward County's emergency communications equipment in the interest of public safety to the residents of the County.

4. Towers shall either maintain a galvanized steel finish or, subject to any applicable standards of the FCC or FAA, be painted a neutral color.

Applicant accepts and acknowledges this provision. As shown on sheet SP-3 of the Site Plan and Drawings in Exhibit "1", the proposed facility will have a galvanized steel finish.

5. At any tower site, the design of the buildings and related structures shall use materials, colors, textures, screening, and landscaping that will blend the facilities to the natural setting and the built environment. The related unmanned equipment structure shall not contain more than 750 square feet of gross floor area or be more than 12 feet in height, and shall be located in accordance with the requirements of the zoning district in which located.

Applicant accepts and acknowledges this provision.

6. Towers shall not be artificially lighted, unless required by the FCC or FAA. If lighting is required, the board of supervisors may review the available lighting alternatives and approve the design that would cause the least disturbances to surrounding views.

Applicant accepts and acknowledges this provision. Due to the structure's height, the FAA will require illumination of the proposed facility. Please see the FAA Notice of Proposed Construction or Alteration Off-Airport and Airspace Analysis by Ken Patterson Airspace Consulting attached hereto as Exhibit "3" and incorporated herein by reference. The facility will be located over 9 nautical miles from the Farmville Regional Airport.

7. All towers must meet or exceed current standards and regulations of the FAA, the FCC, and any other agency of the federal government with the authority to regulate towers. If such standards and regulations are changed, then the owners of the tower governed by this section shall bring such structures into compliance with such revised standards as required by above named agencies. Failure to bring a tower into compliance with such

revised standards and regulations as required by above named agencies shall constitute grounds for the revocation of the special use permit, and removal of the tower at the owner's expense.

Applicant accepts and acknowledges this provision. Please see the FCC 854 attached hereto as Exhibit "4" and incorporated herein by reference.

8. The owner of any tower shall ensure that it is constructed and maintained in compliance with standards contained in applicable federal, state, and local building codes and regulations.

Applicant accepts and acknowledges this provision.

9. Each applicant requesting a special use permit for a new tower shall submit two copies of a scaled site plan and a scaled elevation view and other supporting drawing, calculations, and other documentation, signed and sealed by appropriate licensed professionals, showing the location and dimensions of all improvements, including information concerning topography, radio frequency coverage, height requirements, setbacks, drives, parking, fencing, landscaping, easements, adjacent uses, and any other information deemed necessary by the county to assess compliance with the regulations of this ordinance.

   Additionally the applicant shall provide actual photographs of the site from designated relevant views that include a simulated photographic image of the proposed monopole or tower. The photograph with the simulated image shall include the foreground, the mid-ground, and the background of the site.

Applicant accepts and acknowledges this provision. Please see the Site Plans and Drawings in Exhibit "1"; Site Photos attached hereto as Exhibit '5" and incorporated herein by reference; and Photo Simulations attached hereto as Exhibit "6" and incorporated herein by reference.

10. An engineering report, certifying that the proposed tower and site are compatible for co-location with a minimum of three similar users including the primary user, must accompany the application. The applicant shall provide copies of their co-location policy.

Applicant accepts and acknowledges this provision. Please see the Structural Design Report by Virginia Professional Engineer Amy R. Herbst attached hereto as Exhibit "7" and incorporated herein by reference. The proposed facility has been designed for Verizon Wireless, Prince Edward County emergency communications equipment, and at least three additional broadband carriers. In

addition, please see the ANSI/Fall Zone Certification by Virginia Professional Engineer Amy R. Herbst attached hereto as Exhibit "8" and incorporated herein by reference. Herbst certifies the fall zone at 180', thus any collapse would be contained on the Gaunce property. Also, please see the Collocation Policy Letter of Matt Penning of Milestone Towers attached hereto as Exhibit "9" and incorporated herein by reference.

11. Local government access. Owners of towers shall provide the county co-location opportunities without compensation as a community benefit to improve radio communications for county departments and emergency services, provided it does not conflict with the co-location requirements of this section.

    The facility will also host Prince Edward County's emergency communication equipment.

12. In addition to any reasonable application fees established by board of supervisors, the applicant shall be financially responsible for the cost of any professional engineering and or related services that may be procured by the county to independently verify the application information submitted by the applicant.

Applicant accepts and acknowledges this provision.

13. Towers, guys, and accessory facilities must satisfy the minimum zoning district setback requirements for primary structures.

Applicant accepts and acknowledges this provision. Please see sheet Z-1 of the Site Plan and Drawings, in Exhibit "1". The proposed facility and compound easily meet the A-1 district setbacks of 100 ft. in front, 35 ft. on sides; and 70 ft. in the rear from the proposed property's property lines. The proposed facility setbacks are as follows: 192.4' to the front, 1377.7' to the rear, 477.8' to the east side and 456.6' to the west side.

14. Towers shall be enclosed by security fencing not less than six feet high and shall be equipped with an appropriate anti-climbing device.

Applicant accepts and acknowledges this provision. Please see sheet C-1 of the Site Plan and Drawings in Exhibit "1". The 75 ft. by 75 ft. compound will be secured by a 7 ft. fence topped with three strands of barbed wire as an anti-climbing device, for a total height of 8 ft.

15. Tower facilities shall be landscaped with a buffer of plant materials that effectively screens the view of the support buildings from adjacent property. The standard buffer shall consist of a landscaping strip of at least four feet wide outside the perimeter of the compound. Existing mature tree growth and natural land form on the site shall be preserved to the maximum extent possible.

Applicant accepts and acknowledges this provision. Please see sheets L-1 and L-2 of the Site Plan and Drawings in Exhibit "1". Milestone Towers will plant 16 Nellie R. Stevens Hollys and 47 Wax Myrtles in a 10 ft. buffer area along the compound fence line.

16. Any tower that is not operational for a continuous period of 90 days shall be considered abandoned, and the owner of such tower shall remove same within 90 days of receipt of notice from the building official or county administrator notifying the owner of such removal requirement. Removal includes the removal of the tower, all subterranean tower and fence footers, underground cables and support buildings. The buildings may remain with the approval of the landowner. If there are two or more users of a single tower, then this provision shall not become effective until all users cease using the tower. If the tower is not removed per this section, the county may require the landowner to have it removed. In all cases, the site shall be returned as closely as possible to its original conditions.

Applicant accepts and acknowledges this provision. Please see the Tower Removal Letter by Matthew Penning of Milestone Towers attached hereto as Exhibit "10" and incorporated herein by reference.

17. Every applicant for a special use permit for a tower shall, as a condition for the issuance of the special use permit, file with the building official a continuing bond in the penal sum of not less than $10,000.00 and conditioned for the faithful observance of the provisions of this ordinance and all amendments thereto, and of all the laws and ordinances relating to towers, and which shall indemnify and save harmless the county from any and all damages, judgments, costs, or expenses which the county may incur by reason of the removal or the causing to be removed any tower as provided for in this section.

Applicant accepts and acknowledges this provision. The $10,000 bond will be provided with our application for a building permit. Please see the Indemnification Letter by Matthew Penning of Milestone Towers attached hereto as Exhibit "11" and incorporated herein by reference.

In support of our application, we have also included the Legal Description of the Gaunce property attached hereto as Exhibit "12" and incorporated herein by reference; Recorded Deed attached hereto as Exhibit "13" and incorporated herein by reference; and Memorandum of Lease attached hereto as Exhibit "14" and incorporated herein by reference.

Upon review, please let us know if we can provide any additional information or materials in support of our application. I can be reached at (843) 414-9754 or (843) 813-0103.

Thank you so much for all your help with this.

With warmest regards, I am

Very truly yours,

Jonathan L. Yates

JLY:emb
Enclosures

Exhibit “1”

STOCKTON LAKE

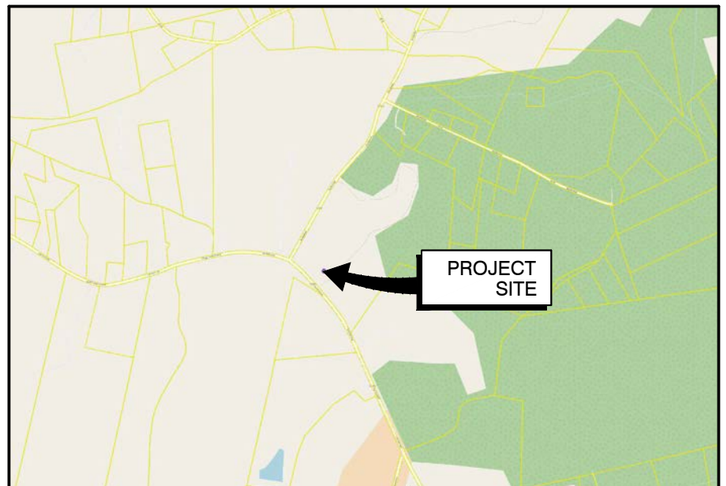
4451 FIVE FORKS ROAD
PAMPLIN, VA 23958

PROJECT DESCRIPTION

INSTALLATION OF NEW 259' SELF SUPPORT TOWER AND FENCED COMPOUND WITHIN A RAWLAND LEASE AREA

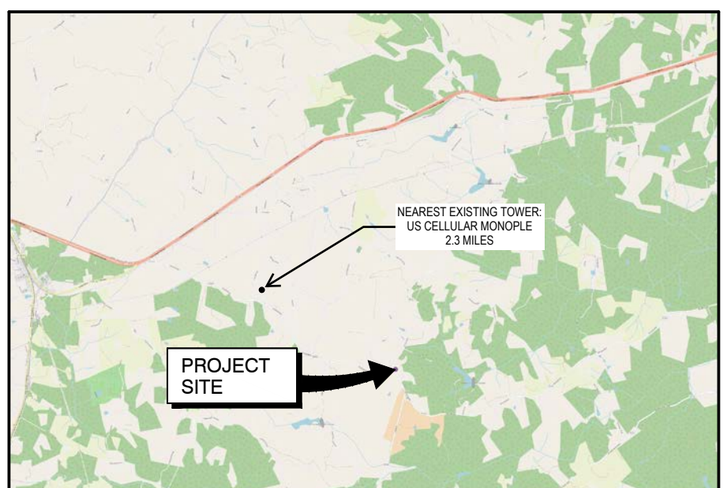
SCAN FOR DIRECTIONS TO SITE:

811 2 WORKING DAYS BEFORE YOU DIG 811 TOLL FREE MISS UTILITY



LOCAL MAP NOT TO SCALE

North



VICINITY MAP NOT TO SCALE

North

PROJECT TEAM

APPLICANT INFORMATION:
MILESTONE TOWERS BTS D/BA MILESTONE TOWERS
12110 SUNSET HILLS ROAD, SUITE 600
RESTON, VA 20190

APPLICANT:	MATT PENNING	PHONE NUMBER:	(703) 865-4697
REAL ESTATE:	SEAN CAI	PHONE NUMBER:	(540) 793-0810
ZONING:	JOHNATHAN YATES	PHONE NUMBER:	(843) 714-4066
CONSTRUCTION:	DAVID TURNER	PHONE NUMBER:	(202) 255-1453
UTILITIES:	DAVID TURNER	PHONE NUMBER:	(202) 255-1453
RF ENGINEER:	MICHELANGELO HOOKER	PHONE NUMBER:	TBD

SUBMITTALS

SUB. NO.	DESCRIPTION	BY	DATE
1	REVIEW SET	SPP	07/19/23
2	ZONING DRAWING SUBMITTAL	SPP	07/31/23
3	SITE PLAN SUBMITTAL	SPP	08/14/23
4	SITE PLAN SUBMITTAL	SPP	9/29/25
5	REVISED SITE PLAN	SPP	12/12/25

CONSULTING TEAM

ENGINEERING:
BAMMAN CONSULTING, LLC
14489 ST ANDREWS LN
ASHLAND, VA 23005
CONTACT: STUART PATTERSON, PE
TELEPHONE: (703) 328-8574

SURVEY:
DICKERSON SURVEYING, LLC
500 COURT ST
APPOMATTOX, VA 24522
CONTACT: MICHAEL RAY GOIN, LS
TELEPHONE: (434) 352-8560

PROJECT SUMMARY

PROPERTY OWNER: ROBERT ALTON GAUNCE

TOWER FACILITY OWNER: MILESTONE TOWERS BTS D/BA MILESTONE TOWERS

UTILITIES INFORMATION:
POWER: SOUTHSIDE ELECTRIC COOPERATIVE
(866) 878-5514
TELEPHONE: KINEX TELECOM, INC.
(434) 392-4804

PROJECT DATA:

ZONING	A-1
PARCEL ID#	046-A-50
ACREAGE	56.25
JURISDICTION	PRINCE EDWARD COUNTY
SITE TYPE	RAW LAND
STRUCTURE TYPE	SELF SUPPORT
STRUCTURE HEIGHT	255'
OVERALL HEIGHT	259' (TOP OF LIGHTNING ROD)
LEASE AREA	100' x 100'
AREA OF LAND DISTURBANCE	7,590 SF (0.174 AC.)

FAA 1A GEOGRAPHIC COORDINATES:

LATITUDE:	37° 14' 33.80" N (NAD 83)	N37.242723095°
LONGITUDE:	78° 34' 45.91" W (NAD 83)	W78.579420417°
ELEVATION:	576.5' AMSL	

ADA COMPLIANCE:
FACILITY IS UNMANNED AND NOT FOR HUMAN HABITATION. SITE WILL NOT BE SERVED BY CITY SEWER OR WATER.

DRAWING INDEX

SHEET TOTAL: 11

14489 ST ANDREWS LN
ASHLAND, VA 23005

Milestone Towers
12110 SUNSET HILLS ROAD, SUITE 600
RESTON, VA 20190

COMMONWEALTH OF VIRGINIA
STUART P. PATTERSON
Lic. No. 041047
February 04, 2026
PROFESSIONAL ENGINEER

SITE INFO.:
STOCKTON LAKE

4451 FIVE FORKS ROAD
PAMPLIN, VA
23958

DESIGN:	SPP
DRAWN:	MAA
REVIEW:	SPP
TTV DATE:	04/19/23
COMM. NO.	-

SUBMITTALS

SYM.	DESCRIPTION	DATE
1	REVIEW SET	7/19/23
2	ZONING DRAWING SUBMITTAL	7/31/23
3	SITE PLAN SUBMITTAL	8/14/23
4	REVISED SITE PLAN	9/29/25
5	REVISED SITE PLAN	12/12/25

SHEET NAME:
COVER SHEET

SHEET NO.:
G-1

LEASE NOTES

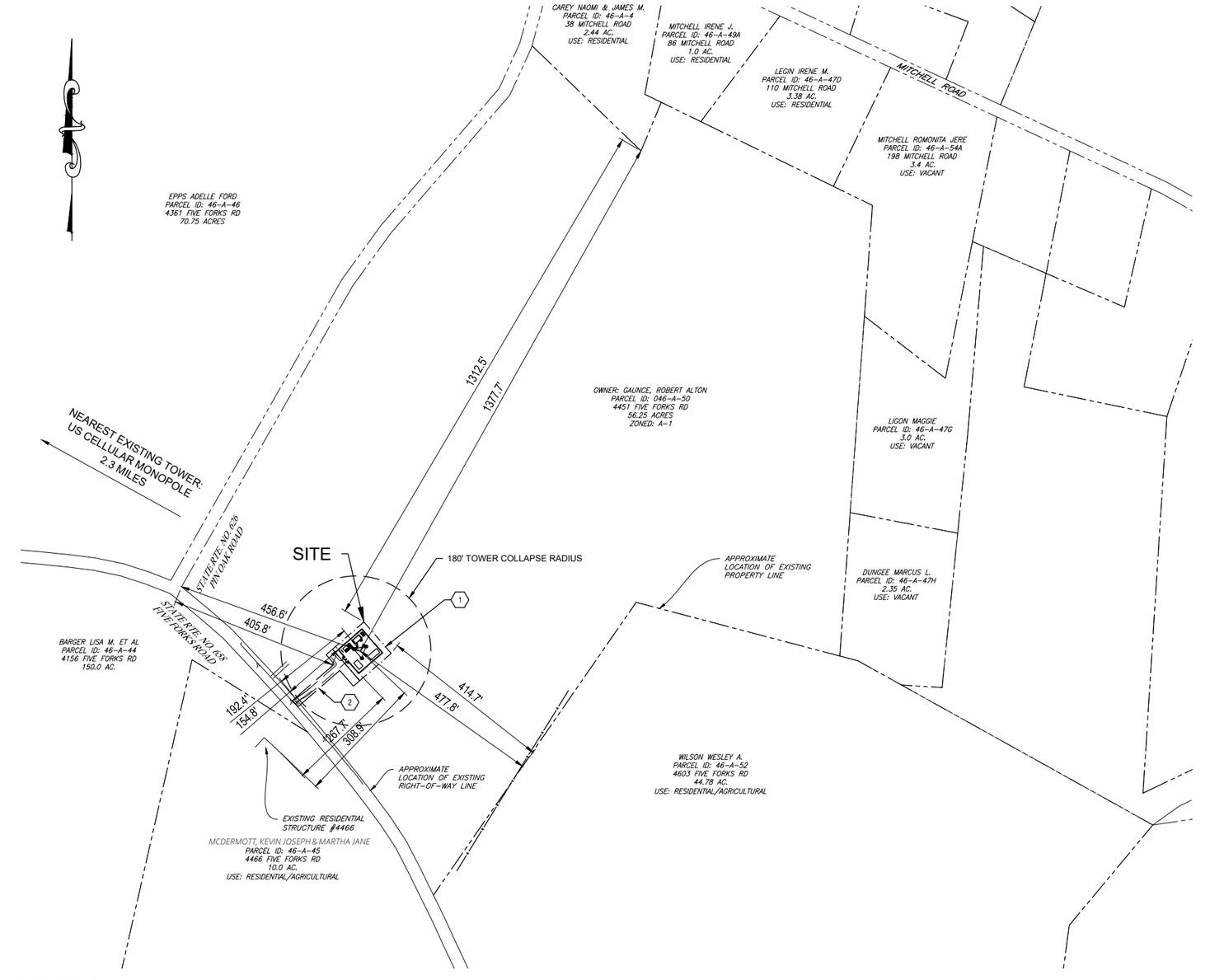
1. 100' x 100' LEASE AREA.
2. 20' WIDE INGRESS/EGRESS AND UTILITY/FIBER EASEMENT.

SETBACK INFORMATION

JURISDICTION: PRINCE EDWARD COUNTY

ZONING: A-1

DIMENSION	REQUIRED	PROPOSED TOWER	PROPOSED COMPOUND
FRONT:	100'	192.4'	154.8'
SIDE:	35'	477.8', 456.6'	405.8', 414.7'
REAR:	70'	1377.7'	1312.5'
EXISTING DWELLING: (4466 FIVE FORKS RD)	N/A	308.9'	267.7'



PARCEL PLAN

GRAPHIC SCALE
150' 0' 150' 300'

14489 ST ANDREWS LN
ASHLAND, VA 23005

Milestone Towers
12110 SUNSET HILLS ROAD, SUITE 600
RESTON, VA 20190

COMMONWEALTH OF VIRGINIA
STUART P. PATTERSON
Lic. No. 041047
February 04, 2026
PROFESSIONAL ENGINEER

SITE INFO.:
STOCKTON LAKE

4451 FIVE FORKS ROAD
PAMPLIN, VA
23958

DESIGN:	SPP
DRAWN:	MAA
REVIEW:	SPP
TTV DATE:	04/19/23
COMM. NO.	-

SUBMITTALS

SYM.	DESCRIPTION	DATE
1	REVIEW SET	7/19/23
2	ZONING DRAWING SUBMITTAL	7/31/23
3	SITE PLAN SUBMITTAL	8/14/23
4	REVISED SITE PLAN	9/29/25

SHEET NAME:
PARCEL PLAN

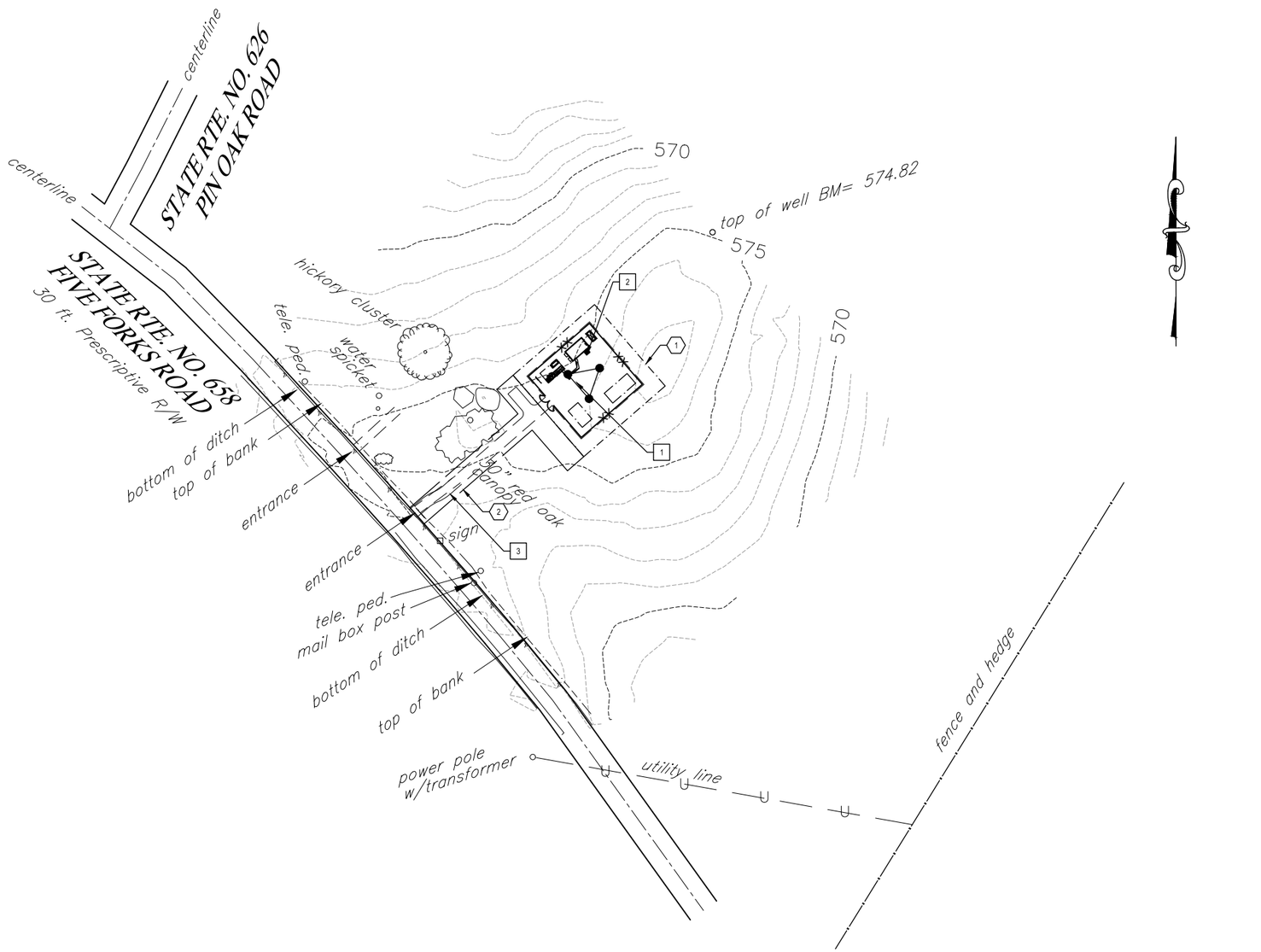
SHEET NO.:
Z-1

LEASE NOTES

1. 100' x 100' LEASE AREA.
2. 20' WIDE INGRESS/EGRESS AND UTILITY/FIBER EASEMENT.

CONSTRUCTION NOTES

1. SELF SUPPORT TOWER, SEE SHEET SP-3.
2. FENCED COMPOUND, SEE SHEET SP-2.
3. ENTRANCE AND ROAD REPAIR/IMPROVEMENTS PER SOW.



SITE PLAN

NOT TO SCALE

GRAPHIC SCALE

60' 0' 60' 120'

GENERAL NOTES

1. THIS PLAN IS SUBJECT TO ALL EASEMENTS AND RESTRICTIONS OF RECORD.
2. THE FACILITY IS UNMANNED AND NOT INTENDED FOR HUMAN HABITATION (NO HANDICAP ACCESS REQUIRED).
3. THE FACILITY IS UNMANNED AND DOES NOT REQUIRE POTABLE WATER OR SANITARY SERVICE.

SURVEYOR'S NOTES

1. PROPERTY LINES SHOWN BASED ON RECORDED DEEDS AND PLATS. THIS IS NOT A BOUNDARY SURVEY.
2. PROJECT IS UNDER THE JURISDICTION OF PRINCE EDWARD COUNTY, VA.
3. SURVEY WAS PERFORMED ON 4/27/23 USING A SOKIA GRX3 RECEIVER WITH TOPCON NETWORK TO ESTABLISH CONTROL. DATA WAS COLLECTED WITH SOKKIA ROBOT.

ABBREVIATIONS

#	NUMBER	N/F	NOW OR FORMERLY
A	AMPS	NO	NUMBER
AGL	ABOVE GROUND LEVEL	OSHA	OCCUPATIONAL SAFETY AND HEALTH ADMINISTRATION
ANSI	AMERICAN NATIONAL STANDARDS INSTITUTE	P/P	POWER POLE
AWG	AMERICAN WIRE GAUGE	RF	RADIO FREQUENCY
CL	CENTERLINE	SYM	SYMBOL
COAX	COAXIAL CABLE	TIA	TELECOMMUNICATIONS INDUSTRY ASSOCIATION
COMM	COMMUNICATION	TTV	TECHNICAL TEAM VISIT
CONC	CONCRETE	TYP	TYPICAL
E	EXISTING, EAST	VA	VIRGINIA
EIA	ELECTRONICS INDUSTRIES ALLIANCE	W/	WITH
GPIN	GRID PARCEL IDENTIFICATION NUMBER	WM	WATER METER
INST	INSTRUMENT	WV	WATER VALVE
MIN	MINIMUM		
NAD	NORTH AMERICAN DATUM		

LEGEND

— - — - —	PROPERTY LINE		CONCRETE
— · — · —	LEASE AREA		
— — — —	EASEMENT		
——//	EDGE OF PAVEMENT		
------90------	CONTOUR		
×90.22	SPOT ELEVATION		
— — — —	GRAVEL		
——XX——	FENCE		
——···——	DITCH		

14489 ST ANDREWS LN
ASHLAND, VA 23005

Milestone Towers
12110 SUNSET HILLS ROAD, SUITE 600
RESTON, VA 20190

COMMONWEALTH OF VIRGINIA
STUART P. PATTERSON
Lic. No. 041047
October 1, 2025
PROFESSIONAL ENGINEER

SITE INFO.:

STOCKTON LAKE

4451 FIVE FORKS ROAD
PAMPLIN, VA
23958

DESIGN:	SPP
DRAWN:	MAA
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TTV DATE:	04/19/23
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SUBMITTALS

SYM.	DESCRIPTION	DATE
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3	SITE PLAN SUBMITTAL	8/14/23
4	REVISED SITE PLAN	9/29/25

SHEET NAME:

SITE PLAN

SHEET NO.:

SP-1

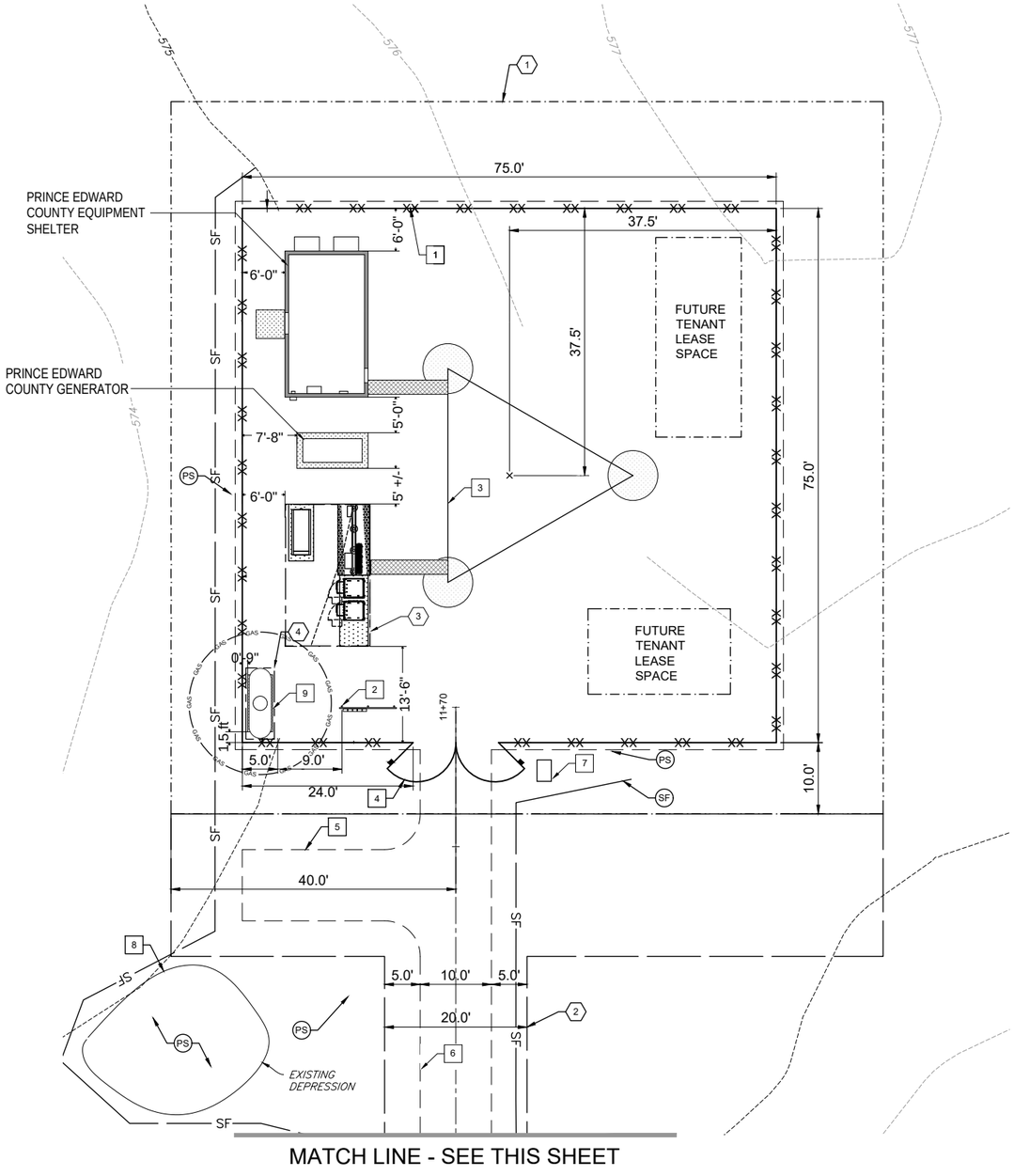
G:\My Drive\BCI WORKING\STOCKTON LAKE - SST\DWG\$WORKING\Sheets\SP\SP-1.dwg Mon, 29 Sep 2025 - 7:40pm marre

STRUCTURAL PRACTICES

SF	SEDIMENT BARRIER	SILT FENCE VA E&SC HANDBOOK STD & SPEC 3.05
CE	SEDIMENT TRANSPORTATION PREVENTION	TEMPORARY CONSTRUCTION ENTRANCE VA E&SC HANDBOOK STD & SPEC 3.02

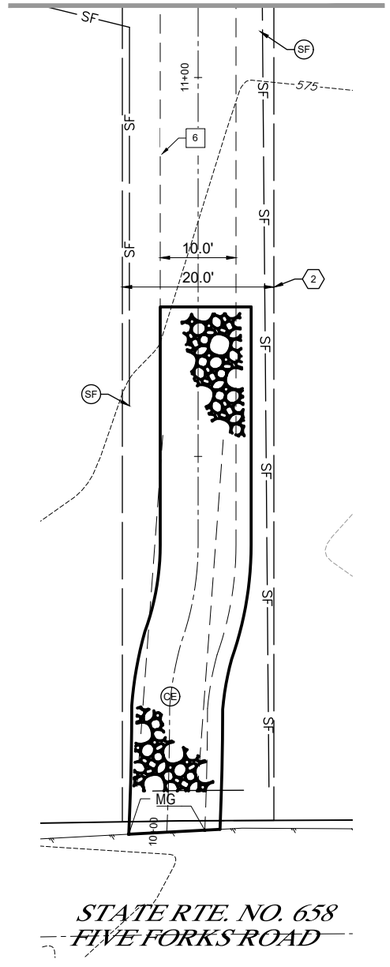
VEGETATIVE PRACTICES

MU	DISTURBED AREA STABILIZATION	MULCHING VA E&SC HANDBOOK STD & SPEC 3.35
TS	DISTURBED AREA STABILIZATION	SEEDING FOR TEMPORARY VEGETATIVE COVER WITH FAST GROWING SEEDINGS ON DISTURBED AREAS THAT WILL NOT BE BROUGHT TO FINAL GRADE WITHIN 30 DAYS VA E&SC HANDBOOK STD & SPEC 3.31
PS	DISTURBED AREA STABILIZATION	PERMANENT VEGETATIVE COVER AFTER FINE GRADING VA E&SC HANDBOOK STD & SPEC 3.32



MATCH LINE - SEE THIS SHEET

MATCH LINE - SEE THIS SHEET



STATE RTE. NO. 658
FIVE FORKS ROAD

COMPOUND PLAN

GRAPHIC SCALE
10' 0' 10' 20'

GENERAL NOTES

1. IT IS THE CONTRACTORS RESPONSIBILITY TO LOCATE EXISTING UNDERGROUND UTILITIES PRIOR TO CONSTRUCTION. PROPOSED UTILITIES INSIDE THE COMPOUND SHALL BE HAND DUG. UTILITY TRENCHES SHALL BE BACKFILLED AND COMPACTED IN 6" LIFTS WITH DETECTABLE WARNING TAPE WITHIN BACKFILLED NATIVE MATERIAL. GEOTEXTILE FABRIC AND STONE SHALL BE RESTORED TO TO EXISTING CONDITIONS.
2. ALL EROSION AND SEDIMENT CONTROL DEVICES SHALL BE INSTALLED BY THE CONTRACTOR IN ACCORDANCE WITH THE LATEST EDITION OF THE VIRGINIA EROSION AND SEDIMENT CONTROL HANDBOOK (VESCH) BEFORE LAND DISTURBANCE COMMENCEMENT AND CHECKED PERIODICALLY TO ENSURE THEY ARE FUNCTIONING AS INDICATED.
3. THE STOCKPILING OF EXCESS MATERIAL ON SITE WILL NOT BE ALLOWED. CONTRACTOR SHALL COORDINATE TO RELOCATION OF EXCESS SOIL WITH THE LANDLORD.
4. ANY VEGETATED AREA DISTURBED BY CONSTRUCTION SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TO SEED AND ESTABLISH A PERMANENT VEGETATIVE COVER.
5. THE CONTRACTOR IS RESPONSIBLE FOR MEANS AND METHODS OF CONSTRUCTION. SAFETY IS THE RESPONSIBILITY OF THE CONTRACTOR. ALL WORK SHALL BE PERFORMED IN ACCORDANCE WITH OSHA REQUIREMENTS.
6. THE CONTRACTOR IS RESPONSIBLE FOR SITE LAYOUT AND CONSTRUCTION STAKING. LOCATION OF EXISTING STRUCTURES AND UTILITIES MUST BE CONFIRMED BY THE CONTRACTOR.
7. ALL WORK SHALL BE ACCOMPLISHED IN ACCORDANCE WITH ALL LOCAL, STATE, AND FEDERAL CODES OR ORDINANCES.
8. THE CONTRACTOR SHALL BE RESPONSIBLE FOR OBTAINING ALL PERMITS AND INSPECTIONS THAT MAY BE REQUIRED BY ANY FEDERAL, STATE, COUNTY OR MUNICIPAL AUTHORITIES.

E&S NOTES

1. EROSION AND SEDIMENT CONTROL SHALL COMPLY WITH COUNTY REQUIREMENTS.
2. REFER TO VIRGINIA EROSION AND SEDIMENT CONTROL MANUAL (3RD EDITION) FOR STRUCTURAL AND VEGETATIVE PRACTICE REQUIREMENTS.
3. TEMPORARY STOCKPILE AREAS SHALL BE SEEDED AFTER CONSTRUCTION.
4. TOTAL LIMITS OF DISTURBANCE = 7,590 SF (0.174 AC.)
5. CONTRACTOR SHALL LAY FILTER CLOTH AND STONE OVER AREA DISTURBED EACH DAY. CONTRACTOR SHALL ALSO SEED AND MULCH STRIPPED AND SPREAD TOPSOIL PRIOR TO LEAVING EACH EVENING.
6. ANY AND ALL MATERIAL OR DEBRIS TRACKED ONTO A PUBLIC OR PRIVATE ROAD SURFACE WILL BE REMOVED THOROUGHLY AT THE END OF EACH DAY. SEDIMENT SHALL BE REMOVED FROM ROADS BY SHOVELING OR SWEEPING AND BE TRANSPORTED TO A SEDIMENT CONTROLLED DISPOSAL AREA.
7. ALL EXCAVATED MATERIAL SHALL BE DISPOSED OF IN A LAWFUL MANNER.
8. NO CITY REFUSE COLLECTION WILL BE PROVIDED TO THIS SITE.

(#)

100' x 100' LEASE AREA.

2. 20' WIDE INGRESS/EGRESS AND UTILITY/FIBER EASEMENT.
3. VERIZON 12' x 20' LEASE AREA.
4. VERIZON 4' x 10' LEASE AREA.

CONSTRUCTION NOTES

1. 7' HIGH CHAIN LINK FENCE WITH 3-STRAND BARBED WIRE.
2. 8' H-FRAME.
3. NEW SELF SUPPORT TOWER. SEE SHEET SP-3.
4. 12' WIDE DOUBLE ACCESS GATE.
5. GRAVEL TURN-AROUND AND PARKING AREA.
6. CONTRACTOR SHALL REPAIR EXISTING GRAVEL ROAD AND ADD BASE AND/OR SURFACE AGGREGATE AS NEEDED TO BRING ROAD IN COMPLIANCE WITH DETAIL ON SHEET SP-3.
7. POWER AND FIBER CONDUIT STUB UPS FROM H-FRAME.
8. TEMPORARY STOCKPILE AREA.
9. 3.5' x 8' CONCRETE PAD & 500 GAL. LP GAS TANK

14489 ST ANDREWS LN
ASHLAND, VA 23005

Milestone Towers
12110 SUNSET HILLS ROAD, SUITE 600
RESTON, VA 20190

COMMONWEALTH OF VIRGINIA
STUART P. PATTERSON
Lic. No. 041047
February 04, 2026
PROFESSIONAL ENGINEER

SITE INFO.:
STOCKTON LAKE

4451 FIVE FORKS ROAD
PAMPLIN, VA
23958

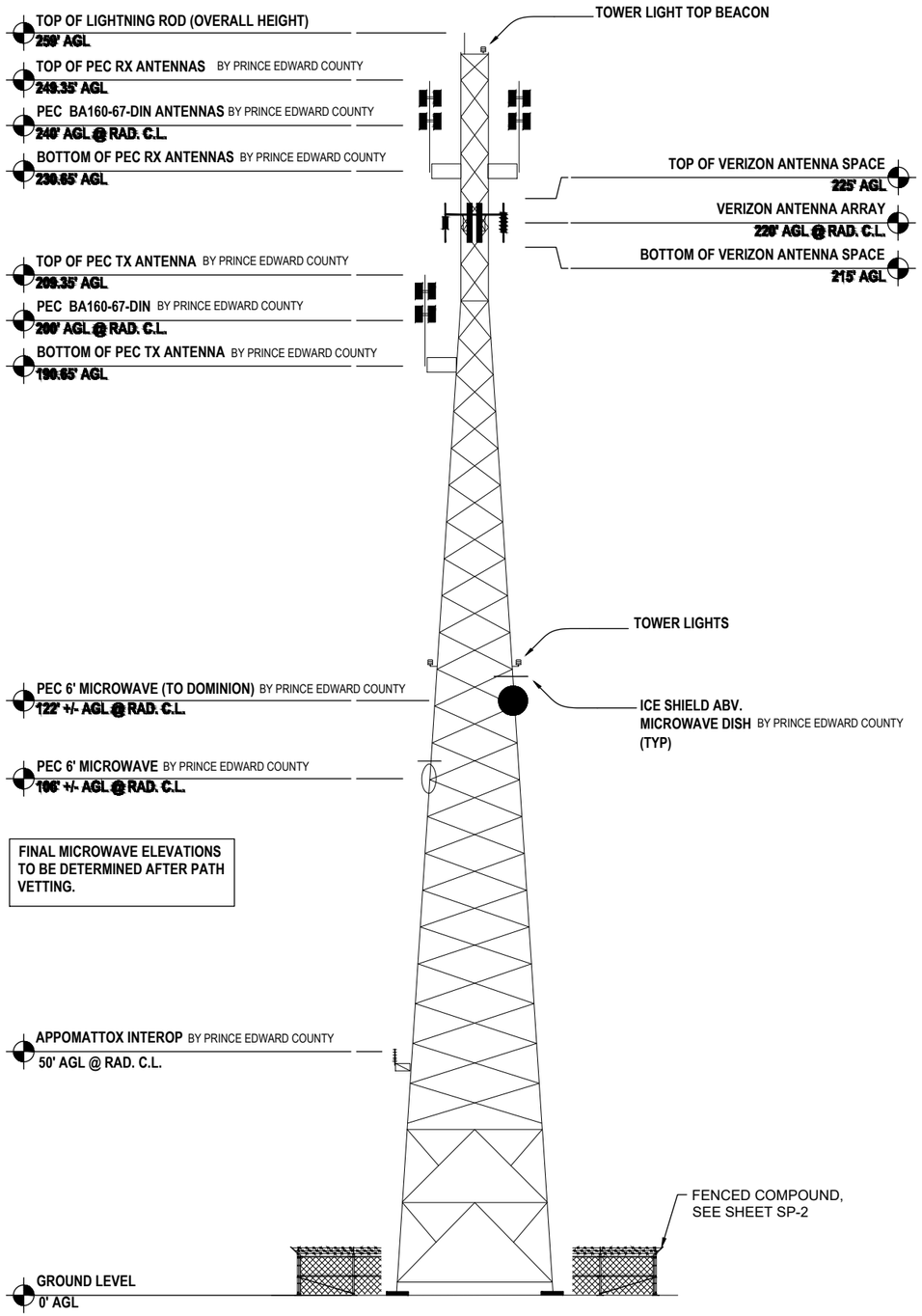
DESIGN:	SPP
DRAWN:	MAA
REVIEW:	SPP
TTV DATE:	04/19/23
COMM. NO.	-

SUBMITTALS

SYM.	DESCRIPTION	DATE
1	REVIEW SET	7/19/23
2	ZONING DRAWING SUBMITTAL	7/31/23
3	SITE PLAN SUBMITTAL	8/14/23
4	REVISED SITE PLAN	9/29/25
5	REVISED SITE PLAN	12/12/25

SHEET NAME:
ENLARGED SITE PLAN

SHEET NO.:
SP-2



ELEVATION

NO SCALE

GENERAL NOTES

1. TRANSMITTER EQUIPMENT AND ANTENNAS ARE DESIGNED TO MEET ANSI/EIA/TIA 222-H REQUIREMENTS.
2. ALL STRUCTURAL ELEMENTS SHALL BE HOT DIPPED GALVANIZED STEEL.
3. SELF SUPPORT AND FOUNDATION DESIGNED BY OTHERS.
4. TOWER SHALL HAVE A GALVANIZED STEEL FINISH.
5. ILLUMINATION OF TOWER IS REQUIRED BY FAA ILLUMINATION STANDARDS.

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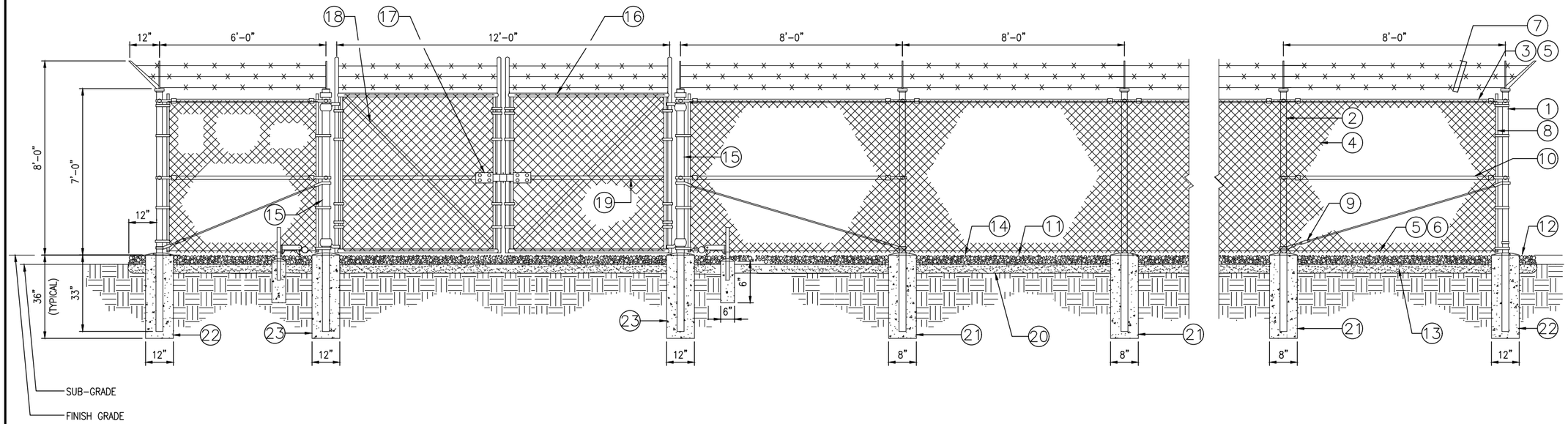
SHEET NAME:
ELEVATION

SHEET NO.:
SP-3



1
C–1

COMPOUND FENCE DETAIL

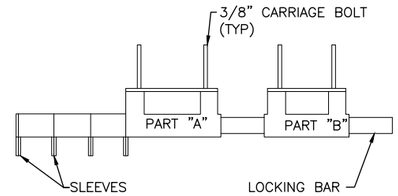
NOT TO SCALE

REFERENCE NOTES:

1. CORNER, END OR PULL POST 3" NOMINAL SCHEDULE 40 PIPE.
2. LINE POST: 2 1/2" SCHEDULE 40 PIPE, PER ASTM–F1083. LINE POSTS SHALL BE EQUALLY SPACED AT MAXIMUM 8'–0" OC
3. TOP RAIL & BRACE RAIL: 1 1/2" PIPE, PER ASTM–F1083.
4. FABRIC: 9 GA CORE WIRE SIZE 2" MESH, CONFORMING TO ASTM–A392.
5. TIE WIRE: MINIMUM 11 GA GALVANIZED STEEL AT POSTS AND RAILS A SINGLE WRAP OF FABRIC TIE AND AT TENSION WIRE BY HOG RINGS SPACED MAX. 24" INTERVALS.
6. TENSION WIRE: 9 GA GALVANIZED STEEL.
7. BARBED WIRE: DOUBLE STRAND 12–1/2" OD TWISTED WIRE TO MATCH WITH FABRIC 14 GA, 4 POINT BARBS SPACED ON APPROXIMATELY 5" CENTERS.
8. STRETCHER BAR.
9. 3/8" DIAGONAL ROD WITH GALVANIZED STEEL TURNBUCKLE OR DIAGONAL THREADED ROD.
10. FENCE CORNER POST BRACE: 1 5/8" DIA EACH CORNER EACH WAY.
11. 1 1/2" MAXIMUM CLEARANCE FROM GRADE.
12. 4" FINISH OR AS DETERMINED BY CONSTRUCTION MANAGER DURING BID WALK.
13. 6" COMPACTED 95% BASE MATERIAL OR AS DETERMINED BY CONSTRUCTION MANAGER DURING BID WALK.
14. FINISH GRADE SHALL BE UNIFORM AND LEVEL.
15. GATE POST 4". SCHEDULE 40 PIPE, FOR GATE WIDTHS UP THRU 7 FEET OR 14 FEET FOR DOUBLE SWING GATE, PER ASTM–F1083.
16. GATE FRAME: 1 1/2" PIPE, PER ASTM–F1083.
17. GATE LOCKING DEVICE (OFCI)
18. 1–1/2 PAIR INDUSTRIAL MALLEABLE IRON OFFSET PIN HINGE (PAGE–WILSON M–6 OR EQUAL)
19. GATE FRAME BRACE
20. GEOTEXTILE FABRIC
21. LINE POST: CONCRETE FOUNDATION (2000 PSI)
22. CORNER POST: CONCRETE FOUNDATION (2000 PSI)
23. GATE POST: CONCRETE FOUNDATION (2000 PSI)

GENERAL NOTES:

1. INSTALL FENCING PER ASTM F–567
2. INSTALL SWING GATES PER ASTM F– 900
3. LOCAL ORDINANCE OF BARBED WIRE PERMIT REQUIREMENT SHALL BE COMPLIED IF REQUIRED.
4. POST & GATE PIPE SIZES ARE INDUSTRY STANDARDS. ALL PIPE TO BE 1 1/2" GALV. (HOT DIP, ASTM A120 GRADE "A" STEEL). ALL GATE FRAMES SHALL BE WELDED. ALL WELDING SHALL BE COATED WITH (3) COATS OF COLD GALV. (OR EQUAL).
5. ALL OPEN POSTS SHALL HAVE END–CAPS.
6. USE GALVANIZED HOG–RING WIRE TO MOUNT ALL SIGNS.
7. ALL SIGNS MUST BE MOUNTED ON INSIDE OF FENCE FABRIC.



1. DRILL 3/8" HOLES IN THE GATE LEAF USING THE GATE DIMENSIONS PROVIDED.
2. SLIDE THE CARRIAGE BOLTS IN THE SLOTS ON THE BACK OF PART "B" AND PUSH THE BOLTS THROUGH THE HOLES DRILLED INT HE GATE FIGURE. PUT THE NUT AND THE LOCKNUT ON AND TIGHTEN AND CUT THE EXCESS BOLT OFF. DO THE SAME WITH PART "A".
3. ADD THE NUMBER OF SLEEVES NEEDED FOR THE NUMBER OF LOCKS AND SLIDE THE LOCKING BAR INTO PLACE THROUGH BOTH PART "A" AND PART "B". NOW INSTALL THE LOCKS.

*IF THE GATE HAS NO CENTER BAR IN THE GATE LEAF YOU MAY NEED TO MOUNT THE STYMIELOCK VERTICALLY USING THE SAME DIMENSIONS GIVEN ON THE GATE FACE.

*VERTICAL APPLICATION MAY ALSO BE USED ON SLIDING GATES WITH MULTIPLE LOCKS.

STYMIELOCK INSTALLATION

NO SCALE

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COMMONWEALTH OF VIRGINIA
STUART P. PATTERSON
Lic. No. 041047
October 1, 2025
PROFESSIONAL ENGINEER

SITE INFO.:

STOCKTON LAKE

4451 FIVE FORKS ROAD
PAMPLIN, VA
23958

DESIGN:	SPP
DRAWN:	MAA
REVIEW:	SPP
TTV DATE:	04/19/23
COMM. NO.	-

SUBMITTALS

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SHEET NAME:

SECURITY FENCE DETAILS

SHEET NO.:

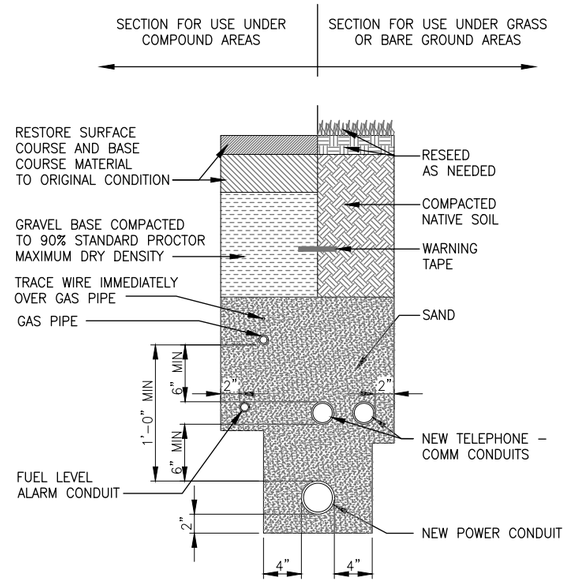
C-1

MINIMUM UTILITY DEPTHS:
(ADDITIONAL DEPTH MAY BE REQUIRED TO MEET MINIMUM CLEARANCES FROM OTHER UTILITIES.)

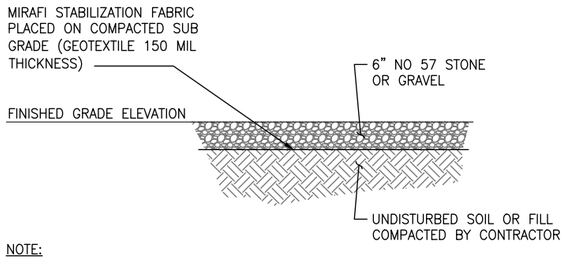
POWER:	36"
COMMUNICATIONS AND ALARMS:	18"
LP GAS PIPING:	12"

NOTE:

1. IF FREE OF ORGANIC OR OTHER DELETERIOUS METERIAL, EXCAVATED MATERIAL MAY BE USED FOR BACKFILL. IF NOT, PROVIDE CLEAN, COMPACTIBLE MATERIAL. COMPACT IN 8" LIFTS. REMOVE ANY LARGE ROCKS PRIOR TO BACKFILLING, CONTRACTOR TO VERIFY LOCATION OF EXISTING U/G UTILITIES PRIOR TO DIGGING.
2. SAND BACKFILL SHALL BE CLEAN, WASHED AND FREE OF DEBRIS OR ROCKS LARGER THAN 1/8".
3. CONTRACTOR SHALL HAND DIG U/G TRENCHING.
4. DETECTING WIRE SHALL BE BURIED DIRECTLY ABOVE NON-METALLIC PIPING AS INDICATED IN THE CONSTRUCTION DOCUMENTS AND AS DIRECTED BY THE CONSTRUCTION MANAGER.



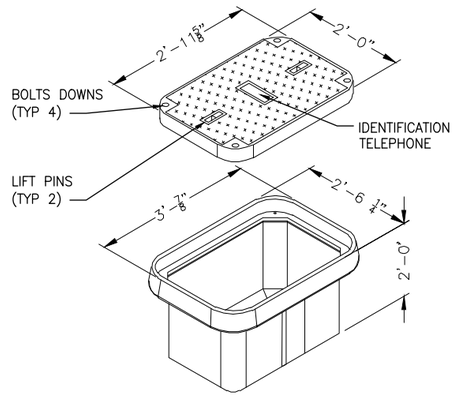
2 / C-2 UTILITY TRENCH DETAIL
NOT TO SCALE



NOTE:

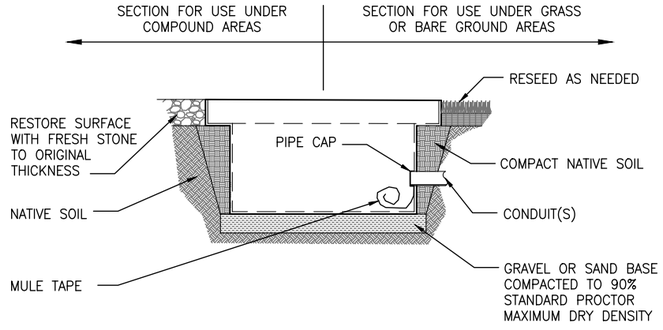
1. FILL SHALL CONSIST OF CLEAN SOIL. NO DELETERIOUS MATERIALS OR ORGANICS TO BE USED.

3 / C-2 GRAVEL COMPOUND DETAIL
NOT TO SCALE



NOTES:

1. ALL STUB-UP CONDUITS INSIDE PULL BOXES WILL BE 6" FROM TOP OF BOX AND HAVE MULE TAPE AND CAPS.
2. EXCAVATION FOR HAND HOLE SHALL BE DONE BY HAND WITHIN FENCE COMPOUND.
3. INSTALLATION SHOULD BE LEVEL AND EVEN WITH SURROUNDING SURFACE AND NOT POSE A TRIP HAZARD.



4 / C-2 HANDHOLE SECTION
NOT TO SCALE

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SHEET NAME:
CONSTRUCTION DETAILS

SHEET NO.:
C-2

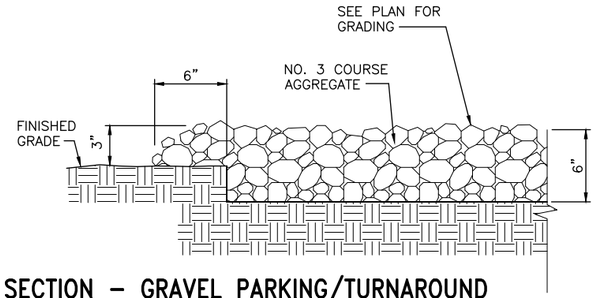
EARTH WORK SUBGRADE COMPACTION & SELECT GRANULAR FILL

Ⓐ CONTRACTOR SHALL BE RESPONSIBLE FOR CLEARING & GRUBBING THE CONSTRUCTION SITE AND ROADWAY AREAS. THE CONTRACTOR SHALL COMPLY WITH THE RECOMMENDATIONS CONTAINED WITHIN THE GEOTECHNICAL REPORT WHEN NECESSARY PREPARED FOR THIS SITE FOR SITE WORK PREPARATION & FOUNDATION WORK. AS A MINIMUM THE TOP 3" OF GRADE SHALL BE REMOVED. THE EXPOSED SUBGRADE COMPACTED, GEOTEXTILE FABRIC AS REQUIRED FOR UNSTABLE SOIL CONDITION.

Ⓑ ALL SELECT GRANULAR FILL SHALL BE COMPACTED TO A 95% COMPACTION AT A MAXIMUM DRY DENSITY AS DETERMINED BY ASTM D-1557 OR WITHIN PLUS OR MINUS 3% OF OPTIMUM MOISTURE CONTENT.

1 / C-3 **TYPICAL ACCESS ROAD CROSS SECTION**

NOT TO SCALE



2 / C-3 **SECTION - GRAVEL PARKING/TURNAROUND (OUTSIDE OF FENCED COMPOUND)**

NO SCALE

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SHEET NAME:
CONSTRUCTION DETAILS

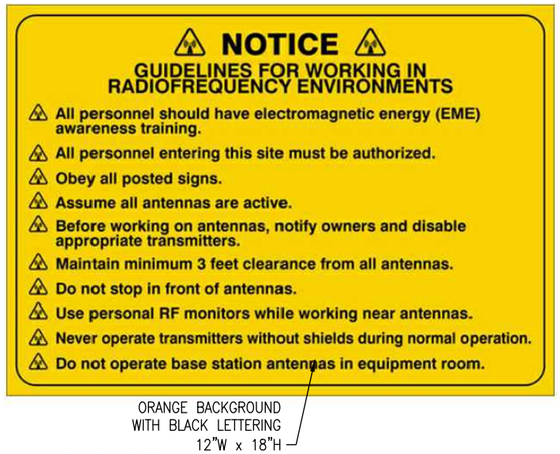
SHEET NO.:
C-3

SIGN TYPES

TYPE	COLOR	COLOR CODE PURPOSE	SIGN MOUNTING LOCATIONS
RF NOTICE	BLUE	**"NOTICE BEYOND THIS POINT"** RF FIELDS BEYOND THIS POINT MAY EXCEED THE FCC GENERAL PUBLIC EXPOSURE LIMIT. OBEY ALL POSTED SIGNS AND SITE GUIDELINES FOR WORKING IN RF ENVIRONMENTS. IN ACCORDANCE WITH FEDERAL COMMUNICATIONS COMMISSION RULES ON RADIO FREQUENCY EMISSIONS 47 CFR-1.1307(b)	FENCE COMPOUND GATE
RF CAUTION	YELLOW	**"CAUTION BEYOND THIS POINT"** RF FIELDS BEYOND THIS POINT MAY EXCEED THE FCC GENERAL PUBLIC EXPOSURE LIMIT. OBEY ALL POSTED SIGNS AND SITE GUIDELINES FOR WORKING IN RF ENVIRONMENTS. IN ACCORDANCE WITH FEDERAL COMMUNICATIONS COMMISSION RULES ON RADIO FREQUENCY EMISSIONS 47 CFR-1.1307(b)	BASE OF TOWER
WARNING	ORANGE/RED	**"WARNING BEYOND THIS POINT"** RF FIELDS AT THIS SITE EXCEED FCC RULES FOR HUMAN EXPOSURE. FAILURE TO OBEY ALL POSTED SIGNS AND SITE GUIDELINES FOR WORKING IN RF ENVIRONMENTS COULD RESULT IN SERIOUS INJURY. IN ACCORDANCE WITH FEDERAL COMMUNICATIONS COMMISSION RULES ON RADIO FREQUENCY EMISSIONS 47 CFR-1.1307(b)	N/A
FACILITY CONTACT SIGN	BLUE/GREEN/WHITE	**"INFORMATIONAL SIGN"** TO NOTIFY OTHERS OF SITE OWNERSHIP AND CONTACT NUMBER.	FENCE COMPOUND GATE
NO TRESPASSING	BLACK/WHITE	**"NO TRESPASSING"** NOTIFICATION THAT THE TOWER FACILITY IS NOT TO BE ACCESSED BY THE PUBLIC.	FENCE COMPOUND GATE
RF NOTICE 2	ORANGE/BLACK	**"NOTICE"** PROVIDE GUIDELINES FOR ALL GUESTS WORKING WITHIN THE TOWER FACILITY AND ON THE TOWER.	FENCE COMPOUND GATE

NOTES:

1. FOR MILESTONE TOWERS LOGO, SEE MILESTONE TOWERS DESIGN SPECIFICATIONS (PROVIDED BY MILESTONE TOWERS).
2. SITE ID SHALL BE APPLIED TO SIGNS USING "LASER ENGRAVING" OR ANY OTHER WEATHER RESISTANT METHOD (MILESTONE TOWERS APPROVAL REQUIRED).
3. TEXT FOR SIGNAGE SHALL INDICATE CORRECT SITE NAME AND NUMBER AS PER MILESTONE TOWERS CONSTRUCTION MANAGER RECOMMENDATIONS.
4. ALL SIGNS WILL BE SECURED WITH ZIP TIES OR STAINLESS STEEL TECH SCREWS.



RF NOTICE SIGN 2

NO SCALE

RF NOTICE SIGN

NO SCALE

RF CAUTION SIGN

NO SCALE

Milestone Towers

IN CASE OF EMERGENCY
CALL: XXX-XXX-XXXX
SITE: NAME

FCC NO. XXXXXXX

LEASING: XXX-XXX-XXX

WHITE BACKGROUND WITH BLACK LETTERING

FACILITY CONTACT SIGN

NO SCALE

NO TRESPASSING SIGN

NO SCALE

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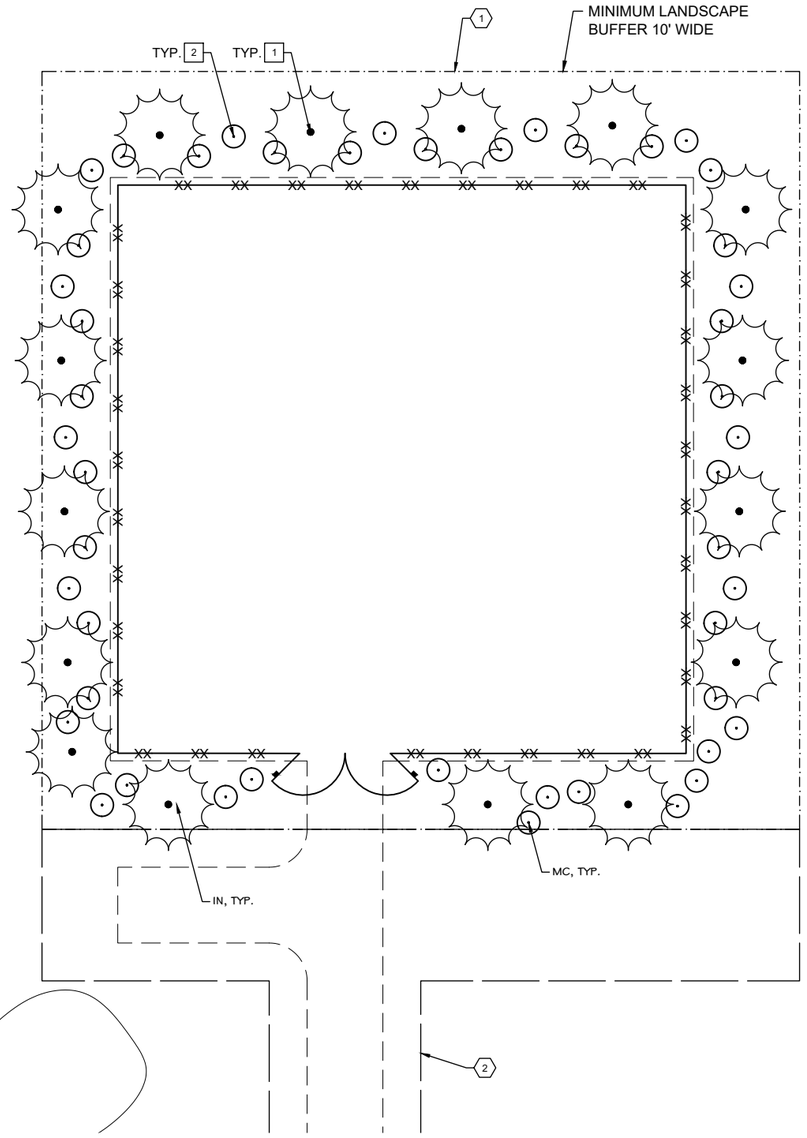
SHEET NAME:

SIGNAGE PLAN

SHEET NO.:

C-4

PLANT SCHEDULE									
SYMBOL	CODE	QTY	REMARKS	BOTANICAL NAME	COMMON NAME	SIZE	CONTAINER	HEIGHT	SPACING
TREES									
	IN	16	PLANT AS SHOWN ON PLANS, MAX 20' SPACING O.C.	Ilex x `Nellie R. Stevens`	Nellie R. Stevens Holly	1" Cal.	B&B/CONT.	5` - 6`	20' MAX.
SHRUBS									
	MC	47	PLANT AS SHOWN ON PLANS, MAX. SPACING 5' O.C.	Myrica cerifera	Wax Myrtle	7 gal.	B&B/CONT.	30" MIN.	60" O.C.



LANDSCAPE PLAN

GRAPHIC SCALE
10' 0' 10' 20'

GENERAL NOTES

1. FOR LEGEND, PROJECT DATA, AND PROJECT NOTES SEE COVER SHEET AND "SP" SHEETS.
2. VEGETATED AREA DISTURBED BY CONSTRUCTION SHALL BE TOPSOILED AND SEEDED TO RESTORE A PERMANENT VEGETATIVE COVER. SEE SEEDING SCHEDULES ON SHEET L-2.
3. IN THE EVENT OF DISCREPANCIES BETWEEN THE PLANTING PLAN AND THE PLANT SCHEDULE, THE PLAN SHALL GOVERN. SUBSTITUTIONS MUST BE COORDINATED WITH THE COUNTY OF GLOUCESTER AND THE OWNER.

SHEET NOTES

1. LANDSCAPE SHEETS ARE FOR LANDSCAPING PURPOSES ONLY. SEE SHEETS FROM OTHER DISCIPLINES, I.E. CIVIL, ELECTRICAL, ARCHITECTURAL, ETC. FOR THE MOST CURRENT INFORMATION PERTAINING TO THOSE DISCIPLINES.
2. AREAS DISTURBED BY CONSTRUCTION NOT OTHERWISE PLANTED OR COVERED IN SITE CONTRACT ARE TO BE SODDED OR SEEDED WITH A STATE CERTIFIED TURF TYPE TALL FESCUE. SEE SHEET L-2 FOR SEEDING SCHEDULES.

(#) LEASE NOTES

1. 100' x 100' LEASE AREA.
2. 20' WIDE INGRESS/EGRESS AND UTILITY/FIBER EASEMENT.
3. VERIZON 12' x 20' LEASE AREA.

CONSTRUCTION NOTES

1. TREE, TYP. SEE PLANT SCHEDULE THIS SHEET. SEE DETAIL, SHEET L-2.
2. SHRUB, TYP. SEE PLANT SCHEDULE THIS SHEET. SEE DETAILS, SHEET L-2.

14489 ST ANDREWS LN
ASHLAND, VA 23005

Milestone Towers
12110 SUNSET HILLS ROAD, SUITE 600
RESTON, VA 20190

COMMONWEALTH OF VIRGINIA
MARTIN A. ARREDONDO
Lic. No. 001794
9/29/25
LANDSCAPE ARCHITECT

SITE INFO.:
STOCKTON LAKE

4451 FIVE FORKS ROAD
PAMPLIN, VA
23958

DESIGN:	SPP
DRAWN:	MAA
REVIEW:	SPP
TTV DATE:	04/19/23
COMM. NO.	-

SUBMITTALS

SYM.	DESCRIPTION	DATE
1	REVIEW SET	7/19/23
2	ZONING DRAWING SUBMITTAL	7/31/23
3	SITE PLAN SUBMITTAL	8/14/23
4	REVISED SITE PLAN	9/29/25

SHEET NAME:
LANDSCAPE PLAN

SHEET NO.:
L-1

PLANTING NOTES

1. CONTRACTOR SHALL GUARANTEE THE QUANTITIES AND PLANT TYPES REQUIRED TO COMPLETE THE LANDSCAPE PLAN AS SHOWN. PLANT SUBSTITUTIONS WILL NOT BE MADE WITHOUT THE WRITTEN CONSENT OF THE OWNER OR THE OWNER'S DESIGNATED REPRESENTATIVE AND THE COUNTY OF GLOUCESTER -- PRIOR TO INSTALLATION.

2. LANDSCAPE MAINTENANCE: IMPROVEMENTS SHOWN HEREON SHALL BE SUBJECT TO REGULAR MAINTENANCE CONSISTING OF, BUT NOT LIMITED TO, FERTILIZATION, PRUNING, REPLACEMENT, INSECT AND DISEASE CONTROL, WATERING, MULCHING, AND WEED CONTROL.

3. PLANTING SEASONS: BALL AND BURLAP PLANT MATERIAL SHALL BE INSTALLED FROM OCTOBER 15 TO MARCH 31. CONTAINER GROWN MATERIAL CAN BE PLANTED YEAR-ROUND. EVERGREEN TREES SHALL BE PLANTED BETWEEN SEPTEMBER 1 AND DECEMBER 1 OR IN SPRING BEFORE NEW GROWTH BEGINS. ANY OTHER DATES NOT LISTED SHALL BE CONSIDERED OUT OF SEASON. PLANT MATERIAL SUBECT TO WARRANTY PERIOD REGARDLESS OF TIME OF INSTALLATION.

4. AREAS DISTURBED BY CONSTRUCTION NOT COVERED BY IMPERVIOUS SURFACE MATERIALS OR PLANT BEDS OR TREE PITS SHALL BE SEEDED UNLESS OTHERWISE NOTED.

5. TREES, SHRUBS, AND GROUND COVERS SHALL BE SPECIMEN QUALITY MEETING THE MINIMUM REQUIREMENTS OF THE AMERICAN ASSOCIATION OF NURSERYMEN STANDARDS, LATEST EDITION, AND SHALL BE NURSERY-GROWN, AND IN A HEALTHY, INSECT AND DISEASE-FREE, CONDITION.

6. TREES AND SHRUBS SHALL BE PLANTED IN ACCORDANCE WITH VIRGINIA COOPERATIVE EXTENSION PUBLICATION 430-295 "TREE AND SHRUB PLANTING GUIDELINES".

7. CONTRACTORS ARE RESPONSIBLE FOR LOCATING AND AVOIDING UTILITIES AND UTILITY EASEMENTS DURING LANDSCAPING OPERATIONS. TREES AND SHRUBS HAVE BEEN LOCATED WITH RESPECT TO NEW OR EXISTING UTILITIES AND EASEMENTS AS ACCURATELY AS POSSIBLE. CONTRACTOR TO FIELD VERIFY THE LOCATIONS OF ALL UTILITIES, UNDERGROUND AND OVERHEAD, PRIOR TO INSTALLING PLANT MATERIAL. CONTACT "MISS UTILITY" OF VIRGINIA AT 811 AT LEAST 48 HOURS BEFORE ANY DIGGING TO AVOID CONFLICT WITH OR DAMAGE TO ANY UTILITIES.

8. CONTRACTOR SHALL PROVIDE A ONE YEAR WARRANTY ON PLANTING MATERIAL AND INSTALLATION.

9. BEFORE MULCHING, WATER PLANTINGS WITH A DEEP SOAKING TO WET THE SOIL 6"-10" DEEP. MAINTAIN WATERING AT A MINIMUM RATE OF ONE INCH PER WEEK FOR THE FIRST MONTH AND A RATE OF ONCE A WEEK FOR REMAINING PORTION OF THE PLANTING SEASON.

10. IF FERTILIZER IS REQUIRED, FERTILIZE ALL PLANT MATERIAL AFTER WATERING WITH A SLOW RELEASE FERTILIZER TO PLANTS PER MANUFACTURER'S DIRECTIONS AT PLANTING. DO NOT FERTILIZE IF SOLUABLE SALT LEVELS ARE CONSIDERED HIGH. MODIFY SOIL CONDITIONS ACCORDINGLY.

11. MULCHING SHALL BE COMPLETED WITHIN 48 HOURS AFTER PLANTING. MULCH SHALL BE SPREAD TO A DEPTH OF 3". OMIT THE COMPACTED EARTH SAUCER AND COVER THE BEDS WITH MULCH AT HEDGE ROWS. PROVIDE A CONTINUOUS BED OF MULCH. GROUPINGS OF SHRUBS SHALL BE INSTALLED IN CONTINUOUS MULCH BEDS.

12. WHERE PLANTING BEDS ABUT WALK AND CURBS, DEPRESS TOPS OF MULCHED BEDS ONE (1) INCH.

13. TEST SOIL PRIOR TO PLANTING TO ASSURE SUITABLE pH LEVEL AND CONDITION FOR PLANTING MATERIAL. AMEND OR REPLACE EXISTING SOIL BASED ON THE FOLLOWING RATES:

    WHERE PLANTS AND/OR TURF GRASSES ARE TO BE ESTABLISHED, TOPSOIL OR A PREPARED SOIL MIXTURE SHALL BE APPLIED AT DEPTHS NOT LESS THAN SIX INCHES FOR GRASSES, TWELVE INCHES FOR SHRUBS, GROUND COVERS, AND ORNAMENTAL PLANTINGS AND EIGHTEEN INCHES FOR TREES IN AREAS WHERE THE SUBSOIL OR EXISTING SOIL PRESENTS ONE OR MORE OF THE FOLLOWING PROBLEMS:

    A. THE TEXTURE, pH, OR NUTRIENT BALANCE OF THE AVAILABLE SOIL CANNOT BE MODIFIED BY REASONABLE MEANS TO PROVIDE AN ADEQUATE GROWTH MEDIUM FOR THE SPECIFIED PLANTS.

    B. THE SOIL MATERIAL IS TOO SHALLOW TO PROVIDE AN ADEQUATE ROOT ZONE AND TO SUPPLY NECESSARY MOISTURE AND NUTRIENTS FOR PLANT GROWTH.

    C. THE SOIL CONTAINS SUBSTANCES POTENTIALLY TOXIC TO PLANT GROWTH.

    OTHERWISE, THE EXISTING SOIL MIXTURES SHALL BE MODIFIED THROUGH THE USE OF SOIL AMENDMENTS (I.E. PEATMOSS, PERLITE, LIME, GYPSUM, ETC.) TO PROVIDE AN ADEQUATE GROWING MEDIUM.

13. TOPSOIL OR PREPARED SOIL MIXTURES SHALL BE FRIABLE AND LOAMY, FREE OF DEBRIS, TRASH, STUMPS, ROCKS, ROOTS, NOXIOUS WEEDS, AND SUBSTANCES POTENTIALLY TOXIC TO PLANTS.

14. TOPSOIL AND PREPARED SOIL MIXTURES SHALL MEET OR EXCEED THE FOLLOWING MINIMUM CRITERIA.

    A. SOIL SOLIDS SHALL CONSIST OF 40-45 PERCENT SAND (NOT TO EXCEED 25% FINE SAND CONTENT), 40 PERCENT SILT, 10 PERCENT CLAY, AND 5-10 PERCENT ORGANIC MATTER.

    B. pH LEVEL SHALL BE ADJUSTED AS NECESSARY TO MATCH THE SPECIFIC pH NEEDS OF THE PROPOSED PLANTS. A pH LEVEL OF 5.5 TO 6.5 IS GENERALLY ACCEPTABLE FOR MOST PLANTING MATERIAL.

    C. SOILS WITH A HIGH LEVEL OF SOLUABLE SALTS SHALL BE AMENDED TO A LEVEL SUITABLE FOR THE SELECTED PLANT MATERIALS TO ESTABLISH PROPER ROOT GROWTH AND VITALITY.

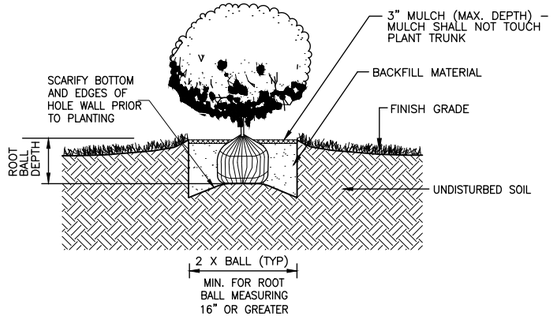
15. GROUPINGS OF PLANTS SHALL BE MULCHED IN CONTINUOUS BEDS.

16. TREES SHALL NOT BE STAKED UNLESS SPECIFICALLY NOTED ON THE PLANTING PLAN. SEE DETAIL SHEET 10, IF STAKING IS NECESSARY.

17. ALL INSTALLED PLANT MATERIAL SHALL BE SUBJECT TO REGULAR MAINTENANCE, INCLUDING FERTILIZATION, PRUNING, REPLACEMENT, INSECT AND DISEASE CONTROL, WATERING, MULCHING, AND WEED CONTROL.

18. FOUNDATION PLANTINGS SHALL BE INSTALLED A MINIMUM OF 3' FROM THE FACE OF THE BUILDING.

19. BUFFER PLANTINGS SHALL BE INSTALLED SO THAT MATURE SHRUB GROWTH WILL NOT EXTEND BEYOND THE PROPERTY LINE OR RIGHT-OF-WAY LINE INTO WALWAYS, AISLES, CREATING AN IMPEDIMENT.



SHRUB PLANTING

NOT TO SCALE

TABLE 3.32-E
(Revised June 2003)
PERMANENT SEEDING SPECIFICATIONS FOR PIEDMONT AREA

(PS)

SEED		
LAND USE	SPECIES	APPLICATION PER ACRE
Minimum Care Lawn (Commercial or Residential)	Tall Fescue Perennial Ryegrass Kentucky Bluegrass	95-100% 0-5% 0-5% TOTAL: 175-200 lbs.
High-Maintenance Lawn	Tall Fescue	TOTAL: 200-250 lbs.
General Slope (3:1 or less)	Tall Fescue Red Top Grass or Creeping Red Fescue Sesional Nurse Crop	128 lbs. 2 lbs. 20 lbs. Total: 150 lbs.
LOW-Maintenance Slope (Steeper Than 3:1)	Tall Fescue Red Top Grass or Creeping Red Fescue Sesional Nurse Crop Crownvetch	108 lbs. 2 lbs. 20 lbs. 20 lbs. Total: 150 lbs.

1- When selecting varieties of turfgrass, use the Virginia Crop Improvement Association (VCIA) recommended turfgrass variety list. Quality seed will bear a label indicating that they are approved by VCIA. A current turfgrass variety list is avalible at the local County Extention office or through VCIA at 804-746-4884 or at http://sudan.cses.vt.edu/html/Turf/turf/publications/publications2/html

2- Use seasonal nurs crop in accordance with seedind dates as ststed below:

February 16th - April.....	Annual Rye
May 1st - August 15th.....	Foxtail Millet
August 16th - October.....	Annual Rye
November - February 15th.....	Winter Rye

3- Substitute Sericea Lespedeza for Crownvetch east of Farmville, VA (May through September use hulled seed, all other periods, use unhulled Sericea). If Flatpea is used, increase to 30 lbs/acre. If weeping lovegrass is used, including in any slope or low maintenance mixture during warmer seeding periods, increase to 30-40 lbs/acre.

FERTILIZER & LIME

- Apply 10-20-10 fertilizer at a rate of 500 lbs./acre (or 12 lbs./1,000 sq. ft.)
- Apply Pulverized Agricultural Limestone ot a rate of 2 tons/acre (or 90 lbs./1,000 sq. ft.)

NOTE:
-A soil test is necessary to determine the actual amount of lime required to adjust the soil pH of the site.
-Incorporate the lime and fertilizer into rhe top 4-6 inches of the soil by disking or by other means.
-When applying Slowly Avalible Nitrogen, use rates avalible in Erosion & Sediment Controle Technical Bulletin #4, 2003 Nutrient Management for Development Sites at http://www.dcr.state.va.us/sw/e&s.html#pubs

TABLE 3.31-B
(REVISED JUNE 2003)
TEMPORARY SEEDING SPECIFICATIONS
QUICK REFERENCE FOR ALL REGIONS

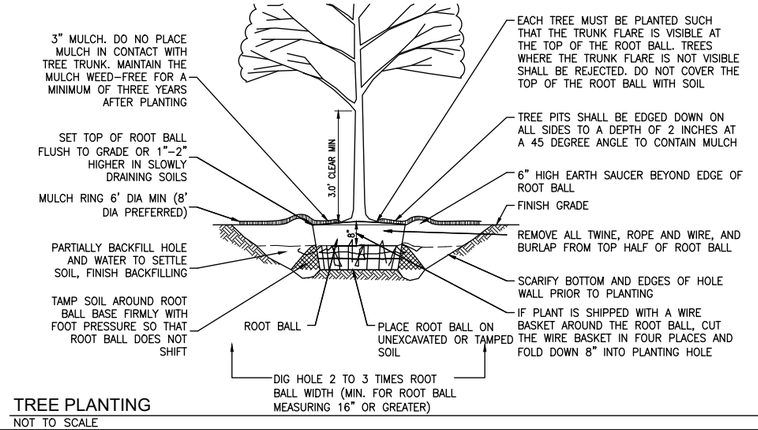
(TS)

SEED		
APPLICATION DATES	SPECIES	APPLICATION RATES
SEPT. 1 - FEB. 15	50/50 MIX OF ANNUAL RYEGRASS (LOLIUM MULTI-FLORUM) & CEREAL (WINTER) RYE (SECALE CEREALE)	50-100 (LBS/ACRE)
FEB. 16 - APR. 30	ANNUAL RYEGRASS (LOLIUM MULTI-FLORUM)	60-100 (LBS/ACRE)
MAY 1 - AUG. 31	GERMAN MILLET	50 (LBS/ACRE)

FERTILIZER AND LIME

- APPLY 10-10-10 FERTILIZER AT A RATE OF 500LBS/ACRE (OR 10LBS/1,000 SQFT)
- APPLY PULVERIZED AGRICULTURAL LIMESTONE AT A RATE OF 2 TONS/ACRE (OR 90LBS/1,000 SQFT)

NOTE:
-A SOIL TEST IS NECESSARY TO DETERMINE THE ACTUAL AMOUNT OF LIME REQUIRED TO ADJUST THE SOIL pH OF SITE.
-INCORPORATE THE LIME AND FERTILIZER INTO THE TOP 4-6 INCHES OF THE SOIL BY DISKING OR BY OTHER MEANS.
-WHEN APPLYING SLOWLY AVAILABLE NITROGEN, USE RATES AVAILABLE IN EROSION & SEDIMENT CONTROL TECHNICAL BULLETIN #4, 2003 NUTRIENT MANAGEMENT FOR DEVELOPMENT SITES AT http://www.dcr.state.va.us/sw/e&s.htm#pubs



TREE PLANTING

NOT TO SCALE

NOTES:
1. ALL TREES TO HAVE SPECIMEN FORM WITH FULL, DENSE CROWNS.
2. DO NOT HEAVILY PRUNE THE TREE AT PLANTING. DO NOT PRUNE TERMINAL LEADER OR BRANCH TIPS. PRUNE ONLY CROSSOVER LIMBS, CO-DOMINANT LEADERS, AND BROKEN OR DEAD BRANCHES. SOME INTERIOR TWIGS AND LATERAL BRANCHES MAY BE PRUNED; HOWEVER, DO NOT REMOVE THE TERMINAL BUDS OF BRANCHES THAT EXTEND TO THE EDGE OF THE CROWN.
3. DO NOT STAKE OR WRAP TRUNK UNLESS NECESSARY.
4. REMOVE TAGS AND LABELS LEAVING AT LEAST ONE OF EACH SPECIES TAGGED FOR IDENTIFICATION.
5. ALL TOPSOIL SHALL BE RETAINED AND REUSED IN PLANTING HOLES. IN INDIVIDUAL PLANTING PITS USE EXCESS SOIL TO FORM AN EARTH SAUCER AROUND THE PLANT (6" FOR TREES, 4" FOR SHRUBS). SAUCERS SHALL BE THE SAME DIAMETER OF SLIGHTLY LARGER THAN THE ROOT BALL. OMIT EARTH SAUCER IN PLANTING BEDS
6. DO NOT USE PLASTIC BENEATH MULCH AROUND TREES AND SHRUBS.
7. WHEN PLANTING IN SLOW DRAINING SOILS LOOSEN SUBSOIL LAYER AROUND THE ROOTBALL BY SHOVEL TO IMPROVE DRAINAGE.

NOTES:
1. ASSURE THAT THE BEARING SURFACE OF THE PROTECTIVE COVERING OF THE WIRE OR CABLE AGAINST THE TREE TRUNK IS A MINIMUM OF 1/2".
2. REMOVE STAKING AS SOON AS THE TREE HAS GROWN SUFFICIENT ROOTS TO OVERCOME THE PROBLEM THAT REQUIRED THE TREE TO BE STAKED. STAKES SHALL BE REMOVED NO LATER THE END OF THE FIRST GROWING SEASON AFTER PLANTING.
3. TREES NORMALLY DO NOT NEED TO BE STAKED AND STAKING CAN BE HARMFUL TO THE TREE. STAKING SHOULD BE DONE ONLY WITH THE APPROVAL OF THE LANDSCAPE ARCHITECT IF IT IS EXPECTED THAT THE TREE WILL NOT BE ABLE TO SUPPORT ITSELF. THE FOLLOWING ARE REASONS WHY TREES DO NOT REMAIN STRAIGHT.

    -TREES WITH POOR QUALITY ROOT BALLS OR ROOT BALLS THAT HAVE BEEN CRACKED OR DAMAGED. REJECT RATHER THAN STAKE.

    -TREES THAT HAVE GROWN TOO CLOSE TOGETHER IN THE NURSERY, RESULTING IN WEAK TRUNKS. REJECT RATHER THAN STAKE.

    -PLANTING PROCEDURES THAT DO NOT ADEQUATELY TAMP SOILS AROUND THE ROOT BALL. CORRECT THE PLANTING PROCEDURE.

    -ROOT BALLS PLACED ON SOFT SOIL. TAMP SOILS UNDER ROOT BALL PRIOR TO PLANTING.

    -ROOT BALLS WITH VERY SANDY SOIL OR VERY WET CLAY SOIL, STAKING ADVISABLE.

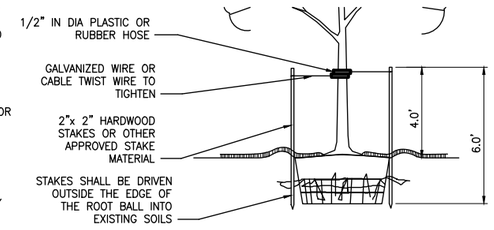
    -TREES LOCATED IN A PLACE OF EXTREMELY WINDY CONDITIONS, STAKING ADVISABLE.

    -TREES WITH LARGE CROWNS, STAKING ADVISABLE.

    -TREES THAT ARE WEAK STEMMED, STAKING ADVISABLE.

WIRE OR CABLE SIZES SHALL BE AS FOLLOWS: TREES UP TO 65 MM (2.5 IN.) CALIPER - 14 GAUGE TREES 65 MM (2.5 IN.) TO 75 MM (3 IN.) CALIPER - 12 GAUGE

TIGHTEN WIRE OR CABLE ONLY ENOUGH TO KEEP FROM SLIPPING. ALLOW FOR SOME TRUNK MOVEMENT. PLASTIC HOSE SHALL BE LONG ENOUGH TO ACCOMMODATE 35MM (1.5 IN.) OF GROWTH AND BUFFER ALL BRANCHES FROM THE WIRE. TUCK ANY LOOSE ENDS OF THE WIRE OR CABLE INTO THE WIRE WRAP SO THAT NO SHARP WIRE ENDS ARE EXPOSED.



TREE STAKING (FOR TREES 3" IN CALIPER OR LESS)

NOT TO SCALE

14489 ST ANDREWS LN
ASHLAND, VA 23005

Milestone Towers
12110 SUNSET HILLS ROAD, SUITE 600
RESTON, VA 20190

COMMONWEALTH OF VIRGINIA
MARTIN A. ARREDONDO
Lic. No. 001794
9/29/25
LANDSCAPE ARCHITECT

SITE INFO.:
STOCKTON LAKE

4451 FIVE FORKS ROAD
PAMPLIN, VA
23958

DESIGN:	SPP
DRAWN:	MAA
REVIEW:	SPP
TTV DATE:	04/19/23
COMM. NO.	-

SUBMITTALS

SYM.	DESCRIPTION	DATE
1	REVIEW SET	7/19/23
2	ZONING DRAWING SUBMITTAL	7/31/23
3	SITE PLAN SUBMITTAL	8/14/23
4	REVISED SITE PLAN	9/29/25

SHEET NAME:
LANDSCAPE DETAILS

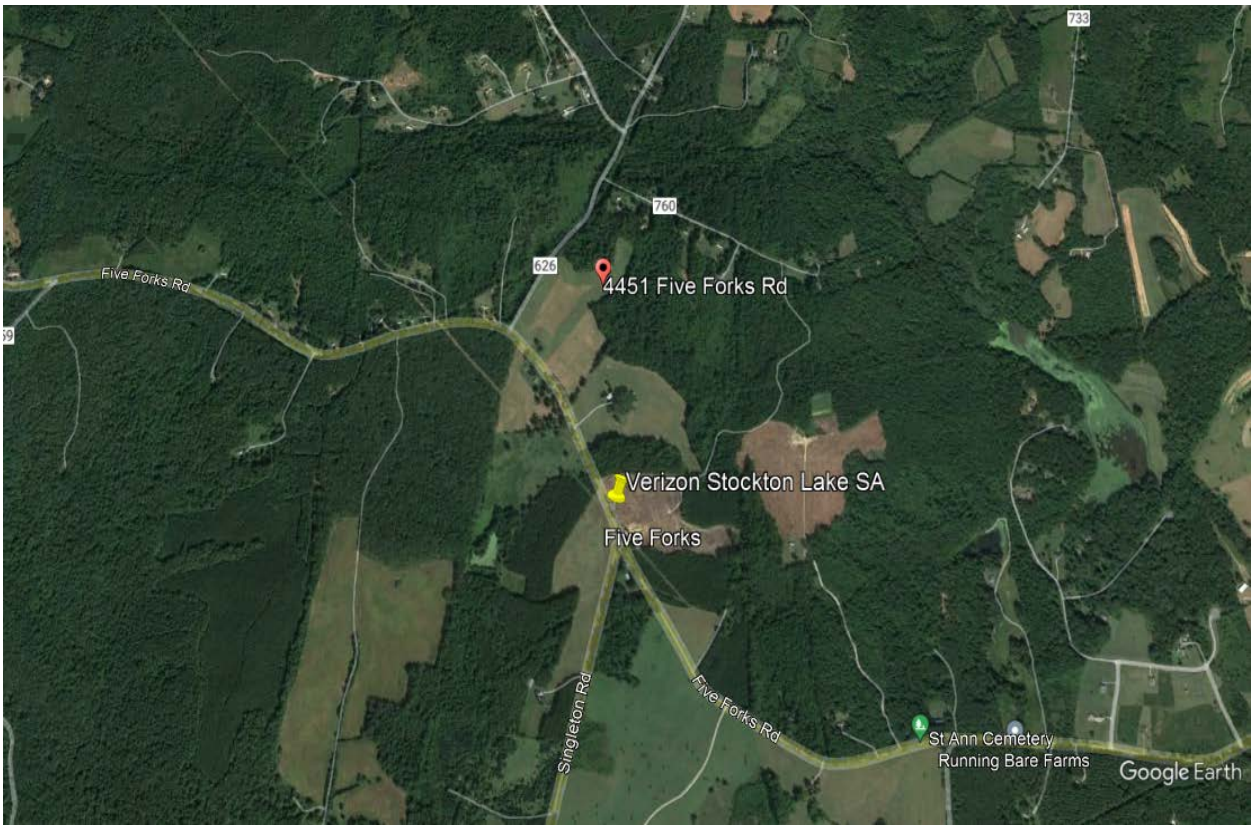
SHEET NO.:
L-2

Exhibit “2”

Milestone Towers: Stockton Lake

Alternative Candidate Analysis

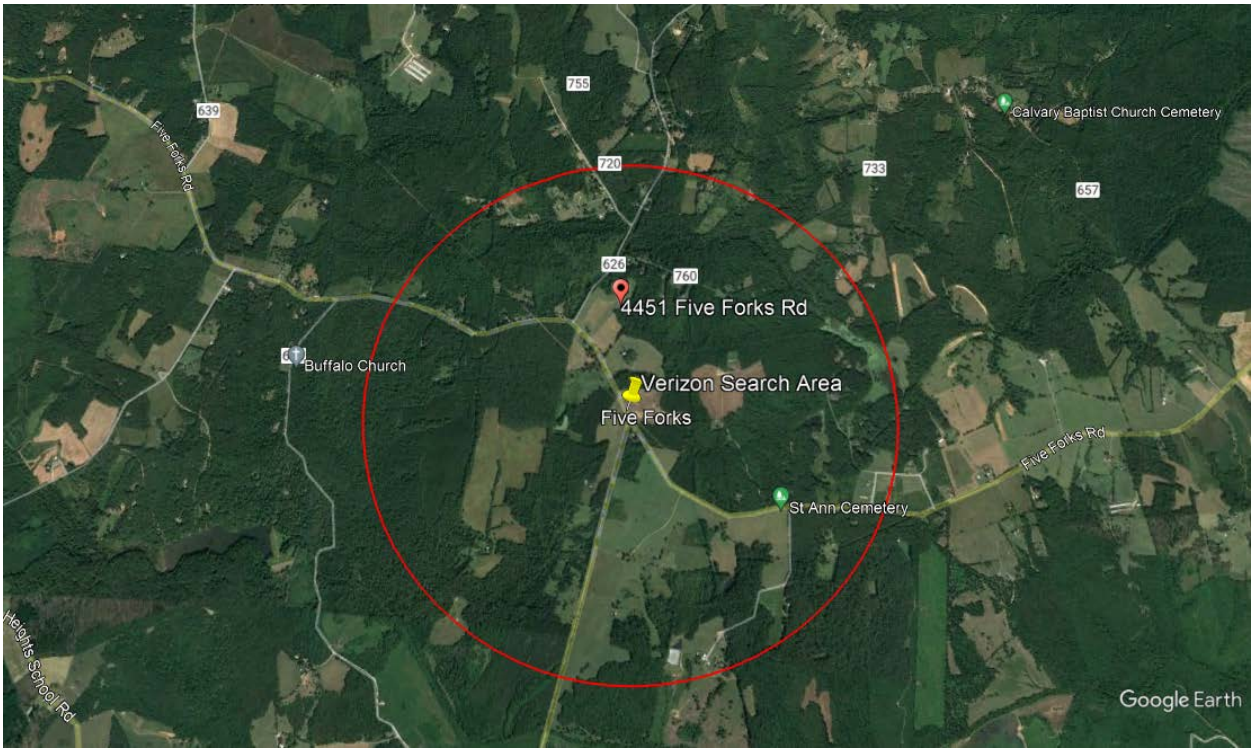
Milestone Towers submits this document to address alternative candidates it considered during its selection process. Milestone Towers has a specific built to suit tower project from Verizon in the area along 4451 Five Forks Road, Pamplin, VA with a requested antenna centerline of 220' on a new 259' tower.



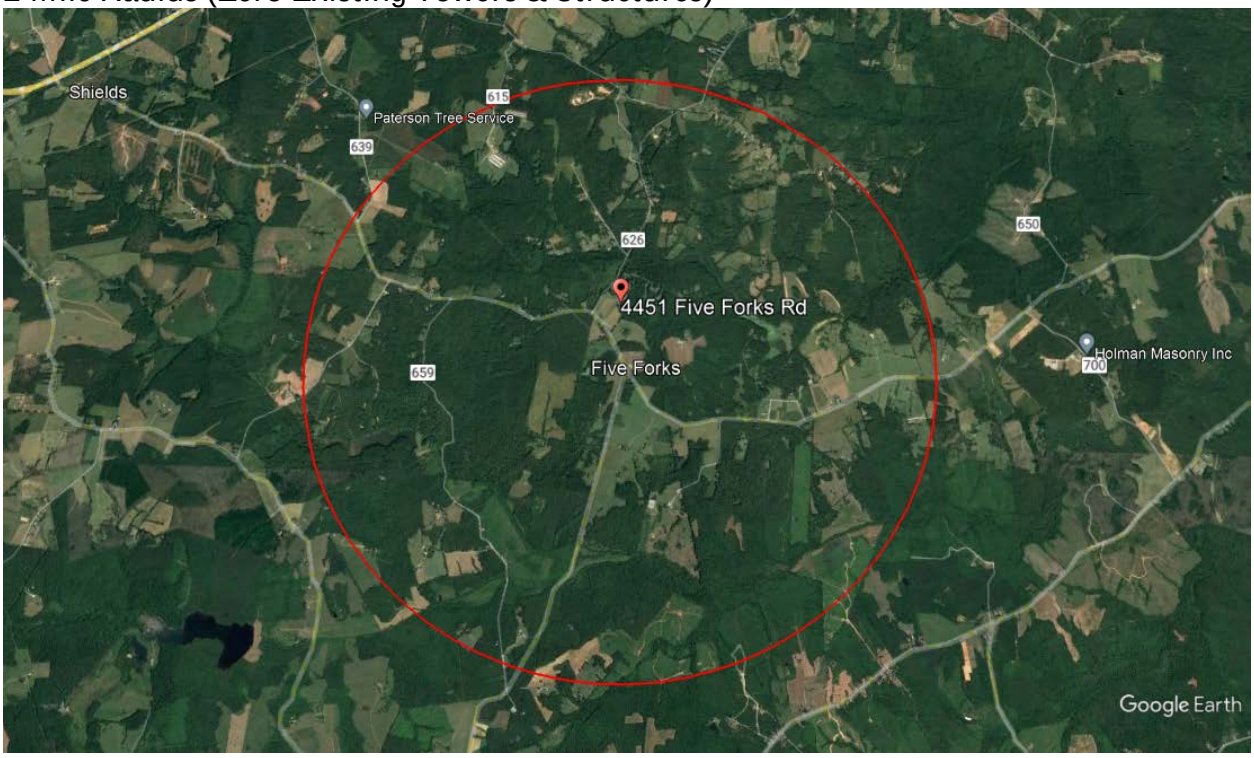
Existing Structures in the Search Ring

Milestone Towers was able to confirm there are no existing towers, or other structures taller than 150 ft. in height within the vicinity of the specified search ring, nor within both a 1 mile & 2 mile radius distance outside of the search ring.

1 Mile Radius (Zero Existing Towers & Structures)

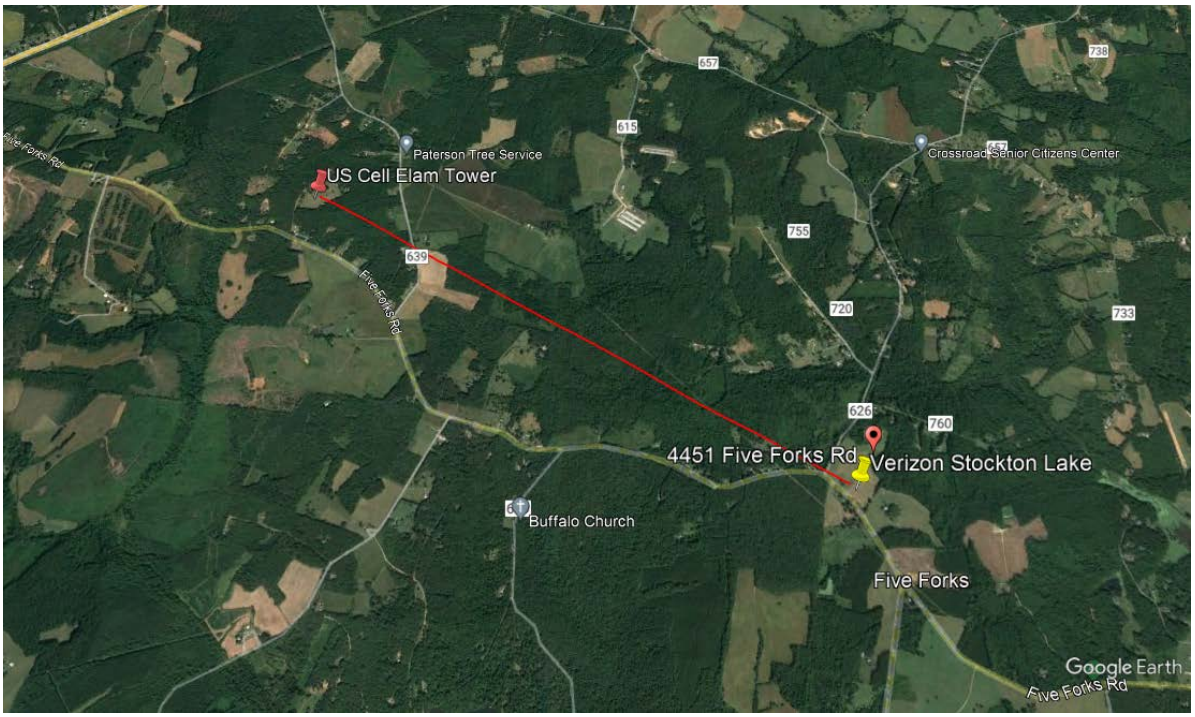


2 Mile Radius (Zero Existing Towers & Structures)

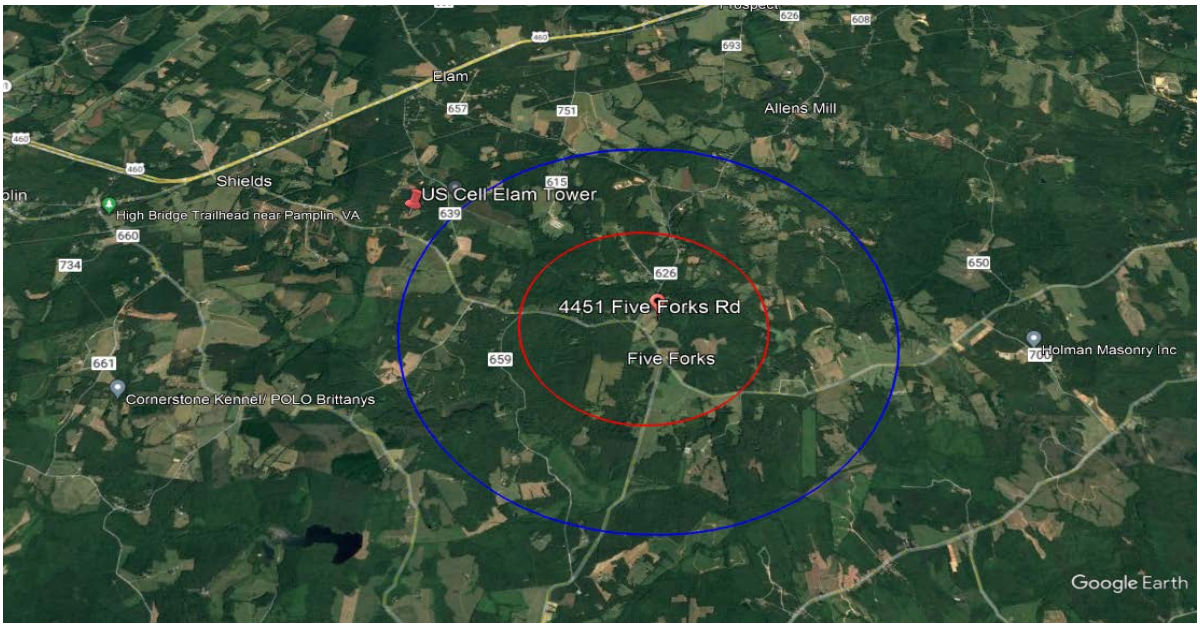


Existing Structures outside of the Search Ring

Milestone Towers then located the nearest existing tower location, (1) one U.S Cellular tower named Elam 2.334 miles NW from the search area & it's too far away to provide any coverage for the search area:



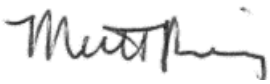
This map illustrates a 1 mile & 2 mile radius from the proposed search ring center, and as shown on the map, there are no existing towers. The closest tower is 2.334 miles to the NW of the search ring and will not cover the intended area.



Conclusion

As noted above, there are no existing structures of sufficient height within the described search ring, and as noted above, the existing tower to the NW is 2.334 miles away and too far to cover the intended coverage area. Milestone Towers has leased a property from Robert Alton Gaunce whose property will be developed in accordance to County regulations.

I certify that the foregoing is true and correct:



_____

Matt Penning, on behalf of Milestone Towers

Exhibit “3”

Home
Dashboard
Reports
Search
Download
Pre-Screening Tool
Tools
FAQ & Resources

File Notice

My Recent Activity

Form Approved OMB No.2120-0001
Expiration Date: 05/31/2026

Privacy Act Statement (5 U.S.C. § 552a(e)(3)): Authority: Information solicited by the Federal Aviation Administration (FAA) Obstruction Evaluation/Airport Airspace Analysis (OE/AAA) is authorized by 49 U.S.C. § 44718 and 47101 Purpose: The FAA OE/AAA is an application used to evaluate all structures that may affect the national airspace system and defend against potential hazards to the safety and efficient use of the navigable airspace. The information collected is used to allow a user access to the OE/AAA and to administer the Aeronautical Study Process. Routine Uses: In accordance with the Privacy Act system of records notice, DOT/ALL 16 Mailing Management System and DOT/FAA 826 Petitions for Exemptions, Other than Medical Exemptions this information may be disclosed to officials within the federal government and the public in general. DOT/ALL 13 - Internet/Intranet Activity and Access Records, this information is routinely used; • To provide information to any person(s) authorized to assist in an approved investigation of improper access or usage of DOT computer systems; • To an actual or potential party or his or her authorized representative for the purpose of negotiation or discussion of such matters as settlement of the case or matter, or informal discovery proceedings; • To contractors, grantees, experts, consultants, detailees, and other non-DOT employees performing or working on a contract, service, grant cooperative agreement, or other assignment from the Federal government, when necessary to accomplish an agency function related to this system of records; and • To other government agencies where required by law.

Disclosure: Submission of the information is voluntary, however, failure to submit requested information will result in FAA's inability to grant you access to the system and may result in an inability of the FAA to process the notice or administer the aeronautical study process for the construction, alteration, activation, or deactivation proposed.

Failure to Provide All Requested Information May Delay Processing of your Notice

U.S. Department of Transportation
Federal Aviation Administration

Notice of Proposed Construction or Alteration

FOR FAA USE ONLY

Aeronautical Study Number
2026-AEA-51-OE

Status: Reviewing

1. Sponsor

Name: MILESTONE TOWERS
Attn of: MATT PENNING
Address: 12110 SUNSET HILLS ROAD
STE 600
City: RESTON State: VA Zip: 20190
Country: US
Phone: tel:+1-1-703-620-2555 Fax:

2. Sponsor's Representative

Name: COLLEEN KHAN
Attn of: ENTREX
Address: 6100 EXECUTIVE BLVD STE 430
City: ROCKVILLE State: MD Zip: 20852
Country: US
Phone: tel:+1-1-202-725-1286 Fax:

3. Notice of: Construction

4. Duration: Permanent (Months: Days:)

5. Work Schedule: N/A

6a. Type: Antenna Tower **6b. Name:** MILESTONE AT STOCKTON LAKE

7. Preferred Marking/Lighting: Not Marked/No Lighting

Current Marking/Lighting:

8. FCC Antenna Registration Number *(if applicable)*:

9. Latitude: See Collected Point(s)

10. Longitude: See Collected Point(s)

11. Datum: See Collected Point(s)

12. Nearest: City: PAMPLIN State:

13. Nearest *Public-use* or Military Airport or Heliport: SOUTHSIDE COMMUNITY HOSPITAL(VA77)

14. Distance from #13. *to Structure*: 55688 ft.

15. Direction from #13. *to Structure*: 247 deg

16. Site Elevation *(SE)*: See Collected Point(s)

17. Structure Height *(AGL)*: See Collected Point(s)

18. Overall Height (#16 + #17) *(AMSL)*: See Collected Point(s)

Current Overall Height (#16 + #17) *(AMSL)*: See Collected Point(s)

19. Previous FAA Aeronautical Study Number *(if applicable)*:

2023-AEA-16270-OE

20. Description of Location:

4451 FIVE FORKS ROAD PAMPLIN, VA 23958

Processed 7460-2 Forms :

Supplemental Form 7460-2 : Add 7460-2

21. Description of

Proposal:
NEW 255' HIGH TOWER PLUS 4' LIGHTNING ROD FOR CELLULAR ANTENNAS

Frequencies:

LOW	HIGH	ERP
806	824 MHz	500 W
824	849 MHz	500 W
851	866 MHz	500 W
869	894 MHz	500 W
896	901 MHz	500 W
901	902 MHz	7 W
930	931 MHz	3500 W
931	932 MHz	3500 W
932	932.5 MHz	17 dBW
935	940 MHz	1000 W
940	941 MHz	3500 W
1850	1910 MHz	1640 W
1930	1990 MHz	1640 W
2305	2310 MHz	2000 W
2345	2360 MHz	2000 W
698	806 MHz	1000 W
6	7 GHz	42 dBW
10	11.7 GHz	42 dBW
17.7	19.7 GHz	42 dBW
21.2	23.6 GHz	42 dBW
806	901 MHz	500 W
929	932 MHz	3500 W
1670	1675 MHz	500 W
1710	1755 MHz	500 W
1850	1990 MHz	1640 W
1990	2025 MHz	500 W
2110	2200 MHz	500 W
2305	2360 MHz	2000 W
2496	2690 MHz	500 W
6	7 GHz	55 dBW
10	11.7 GHz	55 dBW
17.7	19.7 GHz	55 dBW
21.2	23.6 GHz	55 dBW

Specific Frequencies:

LOW	HIGH	ERP
3700	3980 MHz	3280 W

LOW	HIGH	ERP
614	698 MHz	1000 W
614	698 MHz	2000 W

Collected Point(s):

Label	Latitude	Longitude	Datum	AGL	SE	SE Validation	SE Comments
MILESTONE AT STOC...	37 14 33.80N	78 34 45.91W	NAD83	259 ft	577 ft	PASSED	1A survey prepar...

Notice is required by 14 Code of Federal Regulations, part 77 pursuant to 49 U.S.C., Section 44718. Persons who knowingly and willingly violate the notice requirements of part 77 are subject to a civil penalty of $1,000 per day until the notice is received, pursuant to 49 U.S.C., Section 46301(a)

I hereby certify that all of the above statements made by me are true, complete, and correct to the best of my knowledge. In addition, I agree to mark and/or light the structure in accordance with established marking & lighting standards as necessary.

This FAA Form 7460-1 was submitted electronically on 01/06/2026 at 08:14 AM EST.

Add Document

Title	Type	File		
1A SURVEY	Survey	Stockton Lake - FAA Letter - 20251001 (1).pdf	DONE	X

Letter	Date

Previous

Ken Patterson
Airspace Consulting, Inc.
www.airspace-ken.com

Site ID: Stockton Lake

February 6, 2026

Notice to the FAA is Required

To Whom It May Concern:

On February 6, 2026, I personally conducted an evaluation of a proposed telecommunications site for Milestone Towers. The study was to determine if the proposed structure would create any adverse effect on navigable airspace. The site is located near Pamplin, Virginia at 37° 14' 33.80" North and 78° 34' 45.91" West (NAD 83). The site elevation is 577' above mean sea level (AMSL). The proposed structure height is 259' above ground level (AGL) or 836' AMSL. Part 77 of the Federal Air Regulations and Part 17 of the FCC Rules and Regulations were used as the primary reference for this evaluation.

The closest public use or DOD landing surface is Runway 03 at Farmville Regional Airport. The distance to the runway is 9.33 nautical miles on a true bearing of 225.55° from the runway.

The proposed 259' AGL (836' AMSL) structure would exceed FAR Part 77.9 (a)(1) notice requirement by 59'. Notice of Proposed Construction or Alteration Form 7460-1 must be filed with the FAA and a favorable FAA determination must be received prior to beginning construction. The site should be approved by the FAA at the completion of their study.

Any structure exceeding 200' AGL will require marking and/or lighting. The maximum height at this site that would not require notice to the FAA is 200' AGL (777' AMSL).

AM broadcast stations and private use airports are not a factor for this site. For additional information or questions about this study, contact my office anytime.

Sincerely,

Ken Patterson

Ken Patterson

KP15937B

150 Discovery Lake Drive, Fayetteville, GA 30215
(770) 461-0563 kpac0@bellsouth.net

Exhibit “4”

01/15/2026 at 09:44:58
A1358723

FCC Form 854
Main Form

Approved by OMD – 3060-0139
See instructions for public burden estimate

Application for Antenna Structure Registration

Purpose of Filing

1) Enter the application purpose: (**NE**)

AM – Amendment of a Pending Application
AU – Administrative Update
CA – Cancellation of an Antenna Structure Registration
DI – Notification of an Antenna Structure Dismantlement
MD – Modification of a Antenna Structure Registration
NE – Registration of a New Antenna Structure
NT – Required Construction/Alteration Notification
OC – Ownership Change
RE – Registration of a Replacement Antenna Structure
WD – Withdrawal of a Pending Application

2a) If the answer to 1 is AU, CA, DI, MD, NT, OC or RE, provide the FCC Antenna Structure Registration (ASR) Number.	FCC ASR Number:
2b) If the answer to 1 is AM or WD, provide the File Number of the pending application on file.	File Number:
2c) If the answer to 1 is MD or NT, provide the date the Antenna Structure was constructed or the date it was last altered (mm/dd/yyyy).	Date:
2d) If the answer to 1 is DI, provide the date the Antenna Structure was dismantled (mm/dd/yyyy).	Date:

Antenna Structure Ownership Information

3) Select one of the entity types:

( ) Individual ( ) Unincorporated Association ( ) Trust ( ) Government Entity
( ) Corporation (**X**) Limited Liability Company ( ) General Partnership ( ) Limited Partnership
( ) Consortium ( ) Limited Liability Partnership ( ) Other: _____

4) FCC Registration Number (FRN): **0005523832**		5) Assignor FCC Registration Number (FRN):	
6) First Name (if individual):	MI:	Last Name:	Suffix:
7) Legal Entity Name (if not an individual): **Milestone Towers**			
8) Attention To: **Matt Penning**		9) P.O. Box:	**And/Or**
10a) Street Address 1: **12110 Sunset Hills Road Ste. 600**	10b) Street Address 2:		
11) City: **Reston**	12) State: **VA**	13) Zip Code: **20190**	
14) Telephone Number (xxx-xxx-xxxx): **(703) 620-2555**		15) Fax Number: (xxx-xxx-xxxx):	
16) E-mail Address: **matt@milestonetowers.com**			

Contact Representative Information

17) First Name (if individual): **COLLEEN**	MI:	Last Name: **KHAN**	Suffix:
18) Business Name: **ENTREX COMMUNICATION SERVICES, INC.**			
19) Attention To:	20) P.O. Box		**And/Or**
21a) Street Address 1: **6100 EXECUTIVE BLVD STE 430**		21b) Street Address 2:	
22) City: **ROCKVILLE**	23) State: **MD**	24) Zip Code: **20852**	
25) Telephone Number (xxx-xxx-xxxx): **(202) 725-1286**		26) Fax Number: (xxx-xxx-xxxx): **(202) 725-1286**	
27) E-mail Address: **ckhan@entrex.com**			

Antenna Structure Information

28a) Latitude (DD-MM-SS.S): **37- 14- 33.8**		28b) North or South: **North**	
29a) Longitude (DDD-MM-SS.S): **078- 34- 45.9**		29b) East or West: **West**	
30) Street Address or Geographic Location: **4451 FIVE FORKS ROAD**		31) City: **PAMPLIN**	
32) County: **PRINCE EDWARD**	33) State: **VIRGINIA**	34) Zip Code: **23958**	
35) Elevation of site above mean sea level (meters):			**175.7** meters
36) Overall height above ground level (AGL) of the supporting structure without appurtenances:			**77.7** meters
37) Overall height above ground level (AGL) of the antenna structure including all appurtenances:			**78.9** meters
38) Overall height above mean sea level (add items 35 and 37 together):			**254.6** meters

39a) Enter the type of structure on which the antenna will be mounted: (**LTOWER**)

B – Building
BANT – Building with Antenna on Top
BMAST – Building with Mast
BPIPE – Building with Pipe
BPOLE – Building with Pole
BRIDG – Bridge
BTWR – Building with Tower
GTOWER – Guyed Structure Used For Communication Purposes
LTOWER – Lattice Tower
MAST – Mast
MTOWER – Monopole
NNGTANN – Guyed Tower Array
NNLTANN – Lattice Tower Array
NNMTANN – Monopole Array
PIPE – Any type of Pipe
POLE – Any type of Pole
RIG – Oil or Other Type of Rig
SIGN – Any type of Sign or Billboard
SILO – Any type of Silo
STACK – Smoke Stack
TANK – Any type of Tank (water, gas, etc.)
TREE – When used as a support for an antenna
UPOLE – Utility Pole/Tower used to provide service (electric, telephone, etc.)

39b) Number of Towers in Array:	39c) Position of this Tower in the Array:
40a) Array Center Latitude (DD-MM-SS.S):	40b) North or South
41a) Array Center Longitude (DDD-MM-SS.S):	41b) East or West:

Proposed Marking and/or Lighting

42) Enter the proposed marking and/or lighting: (1)
See Form 854 Item 42 Instructions for detailed tier and lighting information.

1) None
2) Paint Only
3) Other _____
4) FAA Style B
5) FAA Style D
6) FAA Style C
7) FAA Style E
8) FAA Style F
9) FAA Style A
10) FAA Style G

FAA Notification

43) FAA Study Number:	44) Date Issued:

Environmental Compliance

45) Does the applicant request a waiver of the Commission's rules for environmental notice prior to construction due to an emergency situation?	(**No**) Yes or No
46a) If the answer to 45 is No, is another federal agency taking responsibility for environmental review of the Antenna Structure?	(**No**) Yes or No
46b) If the answer to 46a is Yes, indicate why: 1) The Antenna Structure is on Federal Land and the landholding agency is taking responsibility for the environmental review of the Antenna Structure. 2) Another federal agency has agreed with the FCC in writing to take responsibility for the environmental review of the Antenna Structure.	( ) 1 or 2
46c) If the answer to 46a is Yes, provide the name of the federal agency taking responsibility for the environmental review of the Antenna Structure.	Name:
47) If the answers to 45 and 46a are No, provide the National Notice Date for the application to be posted on the FCC's website (mm/dd/yyyy).	Date: **06/30/2026**
48) Is the applicant submitting an environmental assessment?	(**No**) Yes or No
49) Does the applicant certify that grant of Authorizations at this location would not have a significant environmental effect pursuant to Section 1.1307 of the FCC's rules?	( ) Yes or No
50) If the answer to 49 is Yes, select the basis for this certification. 1) The construction is exempt from environmental notification (other than due to another agency's review) and it does not fall within one of the categories in Section 1.1307(a) or (b) of the FCC's rules? 2) The construction is exempt from environmental notification due to another agency's review, and the other agency has issued a Finding of No Significant Impact. 3) The environmental notification has been completed, and the FCC has notified the applicant that an Environmental Assessment is not required under Section 1.1307(c) or (d) of the FCC's rules, and the Construction does not fall within one of the categories in Section 1.1307(a) or (b) of the FCC's rules. 4) The FCC has issued a Finding of No Significant Impact.	( ) 1, 2, 3, 4
51) If the answer to 50 is 3 or 4, enter the date that Local Notice was provided (mm/dd/yyyy).	Date:

Certification Statements

1) The applicant certifies that all statements made in this application and in the exhibits, attachments, or documents incorporated by reference are material, are part of this application, and are true, complete, correct, and made in good faith.
2) The applicant certifies that neither the applicant nor any other party to the application is subject to a denial of Federal benefits pursuant to Section 5301 of the Anti-Drug Abuse Act of 1988, 21 U.S.C. § 862, because of a conviction for possession or distribution of a controlled substance. See Section 1.2002(b) of the rules, 47 CFR § 1.2002(b), for the definition of "party to the application" as used in this certification.

Signature (Typed or Printed Name of Party Authorized to Sign) (For OC Applications, to be completed by Assignee)

52) First Name: **COLLEEN**	MI:	Last Name: **KHAN**	Suffix:
53) Title: **AGENT**			
54) Signature: **COLLEEN KHAN**			55) Date: **Jan 15, 2026**

Signature (Typed or Printed Name of Party Authorized to Sign) (For OC Applications, to be completed by Assignor)

56) First Name:	MI:	Last Name:	Suffix:
57) Title:			
58) Signature:			59) Date:

Exhibit “5”


View from site, looking west


View from site, looking north


View from site, looking east


View from site, looking south


View from road, looking towards site

Exhibit “6”

Milestone Towers

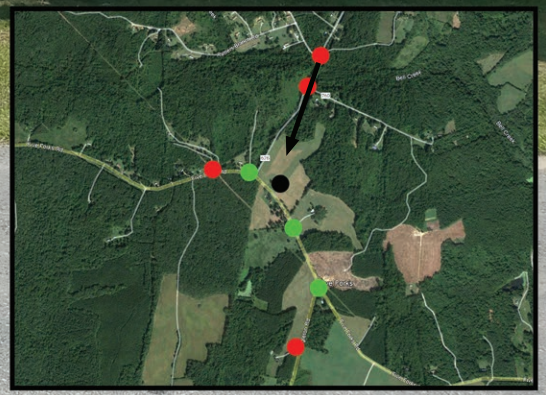
STOCKTON LAKE

4551 Five Forks Road, Pamplin, VA 23958

255ft. SELF SUPPORT

TOWER NOT VISIBLE

View #1 from Rattlers Branch Road
approx. 2,470ft. north-northeast of site



Milestone Towers

STOCKTON LAKE

4551 Five Forks Road, Pamplin, VA 23958

255ft. SELF SUPPORT

TOWER NOT VISIBLE

View #2 from Mitchell Road
approx. 1,880ft. north-northeast of site


Milestone
Towers
STOCKTON LAKE
4551 Five Forks Road, Pamplin, VA 23958
255ft. SELF SUPPORT
TOWER SIMULATION
View #3 from Five Forks Road
approximately 850ft. southeast of site

Milestone Towers

STOCKTON LAKE

4551 Five Forks Road, Pamplin, VA 23958

255ft. SELF SUPPORT TOWER SIMULATION

View #4 from Singleton Road
approximately 850ft. southeast of site

Milestone Towers

STOCKTON LAKE

4551 Five Forks Road, Pamplin, VA 23958

255ft. SELF SUPPORT

TOWER NOT VISIBLE

View #5 from Singleton Road approximately 3,015ft. south of site

Milestone Towers

STOCKTON LAKE

4551 Five Forks Road, Pamplin, VA 23958

255ft. SELF SUPPORT

TOWER NOT VISIBLE

View #6 from Five Forks Road
approx. 1,225ft. west-northwest of site

Milestone Towers

STOCKTON LAKE

4551 Five Forks Road, Pamplin, VA 23958

255ft. SELF SUPPORT

TOWER SIMULATION

View #7 from Five Forks Road
approx. 540ft. west-northwest of site

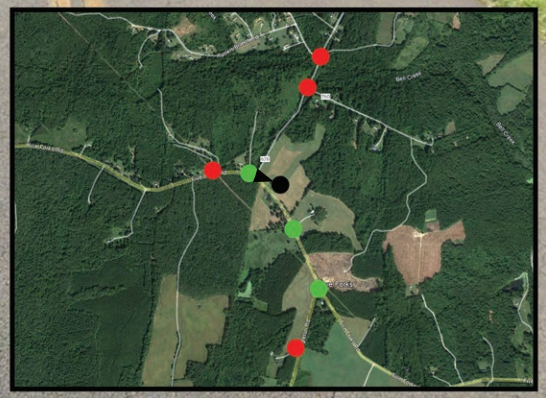


Exhibit “7”

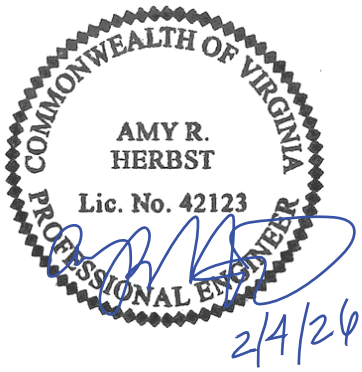
Structural Design Report

255' S3TL Series HD2 Self-Supporting Tower
Site: Stockton Lake, VA

Prepared for: MILESTONE COMMUNICATIONS, INC.
by: Sabre Industries ™

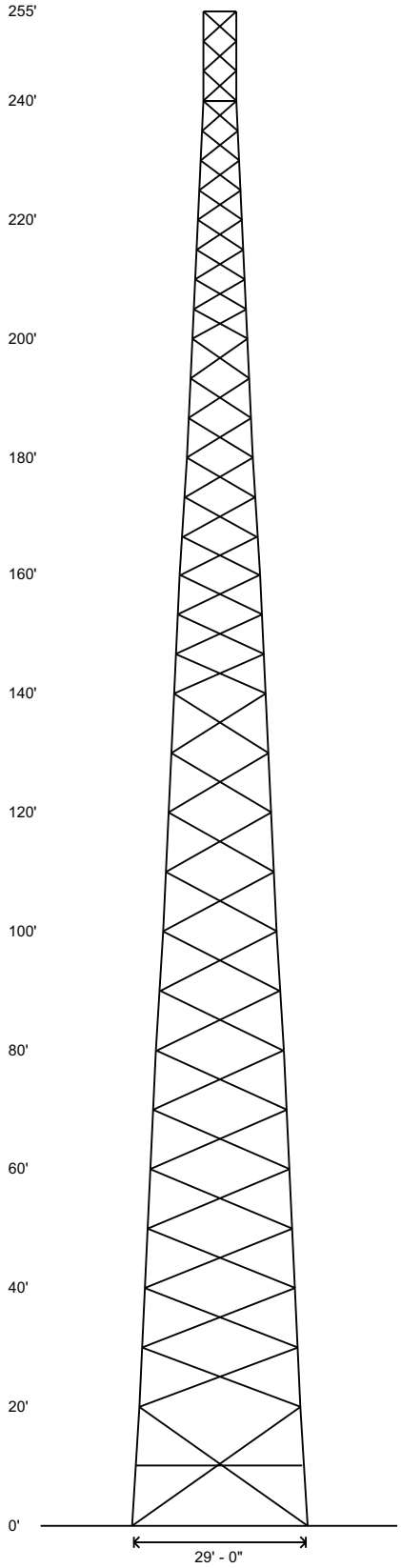
Job Number: 26-3777-TLJ

February 4, 2026



Digitally Signed By Amy R. Herbst
DN: c=US, st=Texas, l=Alvarado, o=SABRE INDUSTRIES, INC., cn=Amy R. Herbst, email=arherbst@sabreindustries.com Date: 2026.02.04 14:31:40

Legs	8.625 OD X .500			8.625 OD X .322		A	B	C	D	E	F	G
Diagonals	L 5 X 5 X 5/16	L 4 X 4 X 1/4		H	I	L 3 X 3 X 3/16	J	L 2 X 2 X 3/16	L 2 X 2 X 1/8			
Horizontals				NONE							K	K
Sub-Horizontals	L 3 X 3 X 1/4			NONE							NONE	K
Brace Bolts	(2) 3/4"	(2) 5/8"		(1) 3/4"						(1) 5/8"		
Top Face Width	27'	25'	23'	21'	19'	17'	15'	13'	11'	9'	7'	5'
Panel Count/Height	1 @ 20'	12 @ 10'						9 @ 6.6667'			11 @ 5'	
Section Weight	5757	5406	5253	5013	3940	3597	3306	2587	2082	1401	1106	906	565



Design Criteria - ANSI/TIA-222-H

Wind Speed (No Ice)	119 mph
Wind Speed (Ice)	30 mph
Design Ice Thickness	1.50 in
Risk Category	III
Exposure Category	C
Topographic Factor Procedure	Method 1 (Simplified)
Topographic Category	1
Ground Elevation	575 ft
Seismic Importance Factor, Ie	1.25
0.2-sec Spectral Response, Ss	0.184 g
1-sec Spectral Response, S1	0.058 g
Site Class	D (DEFAULT)
Seismic Design Category	B
Basic Seismic Force-Resisting System	Telecommunication Tower (Truss: Steel)

Base Reactions - Wind/Ice

Total Foundation		Individual Footing	
Shear (kips)	89.31	Shear (kips)	53.32
Axial (kips)	219.6	Compression (kips)	510
Moment (ft-kips)	12171	Uplift (kips)	445

Base Reactions - Seismic

Total Foundation		Individual Footing	
Shear (kips)	3.03	Shear (kips)	3.05
Axial (kips)	87.14	Compression (kips)	49
Moment (ft-kips)	490	Uplift (kips)	0

Notes

1) All legs are A500 (50 ksi Min. Yield).
2) All braces are A572 Grade 50.
3) All brace bolts are A325-X.
4) The tower model is S3TL Series HD2.
5) Transmission lines are to be attached to standard 8 hole waveguide ladders.
6) Azimuths are relative (not based on true north).
7) Foundation loads shown are maximums.
8) All unequal angles are oriented with the short leg vertical.
9) Weights shown are estimates. Final weights may vary.
10) This tower design and, if applicable, the foundation design(s) shown on the following page(s) also meet or exceed the requirements of the 2021 International Building Code.
11) Tower Rating: 99.55%
12) No grout is required under the base plates.
13) For face widths between 27' and 37', diagonal members are continuous and sub-horizontal members are split.

Sabre Industries
7101 Southbridge Drive
P.O. Box 658
Sioux City, IA 51102-0658
Phone: (712) 258-6690
Fax: (712) 279-0814

Job:	**26-3777-TLJ**
Customer:	MILESTONE COMMUNICATIONS, INC.
Site Name:	Stockton Lake, VA
Description:	255' S3TL
Date:	2/4/2026 By: ARH

Designed Appurtenance Loading

Elev	Description	Tx-Line
238.86	(2) BA160-67-DIN	(2) 1/2"
230.65	(2) 6ft Sidearms	
220	(1) 250 Sq. Ft. EPA (4,500 lbs)	(6) 1 5/8"
198.86	(2) BA160-67-DIN	(2) 1/2"
190.65	(2) 6ft Sidearms	
180	(1) 200 sq. ft. EPA, 4,500 lb Weight	(6) 1 5/8"
170	(1) 150 sq. ft. EPA, 4,500 lb Weight	(6) 1 5/8"
160	(1) 150 sq. ft. EPA, 4,500 lb Weight	(6) 1 5/8"

Elev	Description	Tx-Line
122	Leg Dish Mount	
122	(1) 6' Ice Shield	
122	(1) VHLPX6-6W-6WH	(1) 1 5/8"
106	Leg Dish Mount	
106	(1) VHLPX6-6W-6WH	(1) 1 5/8"
50	6ft Sidearm	
50	(1) RDA6-61	(1) EW63

Material List

Display	Value
A	5.563 OD X .500
B	5.563 OD X .375
C	4.000 OD X .318
D	4.000 OD X .226
E	3.500 OD X .216
F	2.875 OD X .203

Display	Value
G	2.375 OD X .154
H	L 3 1/2 X 3 1/2 X 1/4
I	L 3 1/2 X 3 X 1/4
J	L 2 1/2 X 2 1/2 X 1/4
K	L 2 X 2 X 1/8

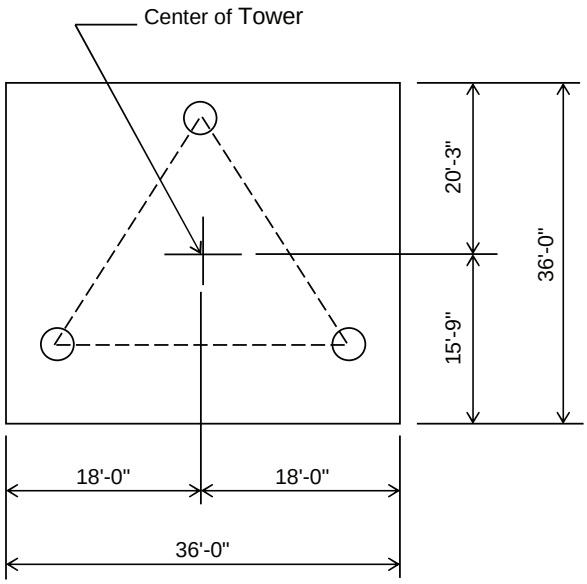
Sabre Industries
7101 Southbridge Drive
P.O. Box 658
Sioux City, IA 51102-0658
Phone: (712) 258-6690
Fax: (712) 279-0814

Job:	**26-3777-TLJ**
Customer:	MILESTONE COMMUNICATIONS, INC.
Site Name:	Stockton Lake, VA
Description:	255' S3TL
Date:	2/4/2026 By: ARH

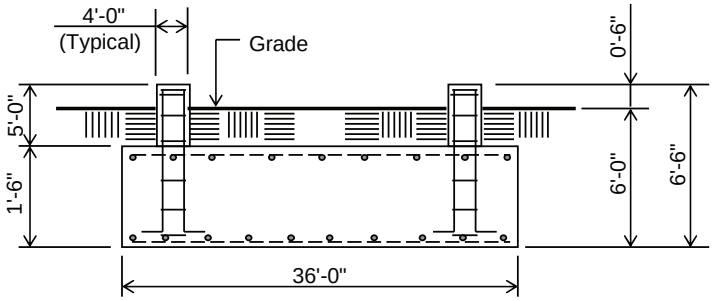
No.: 26-3777-TLJ
Date: 02/04/2026
By: ARH

Customer: MILESTONE COMMUNICATIONS, INC.
Site: Stockton Lake, VA
255 ft. Model S3TL Series HD2 Self Supporting Tower

PRELIMINARY -NOT FOR CONSTRUCTION-



PLAN VIEW



ELEVATION VIEW

(79.0 cu. yds.)
(1 REQD.; NOT TO SCALE)

CAUTION: Center of tower is not in center of slab.

Notes:

1) Concrete shall have a minimum 28-day compressive strength of 4,500 psi, in accordance with ACI 318-14.

2) Rebar to conform to ASTM specification A615 Grade 60.

3) All rebar to have a minimum of 3" concrete cover.

4) All exposed concrete corners to be chamfered 3/4".

5) The foundation design is based on presumptive clay soil as defined in ANSI/TIA-222-H-2017. It is recommended that a soil analysis of the site be performed to verify the soil parameters used in the design.

6) 4.5' of soil cover is required over the entire area of the foundation slab.

7) The bottom anchor bolt template shall be positioned as closely as possible to the bottom of the anchor bolts.

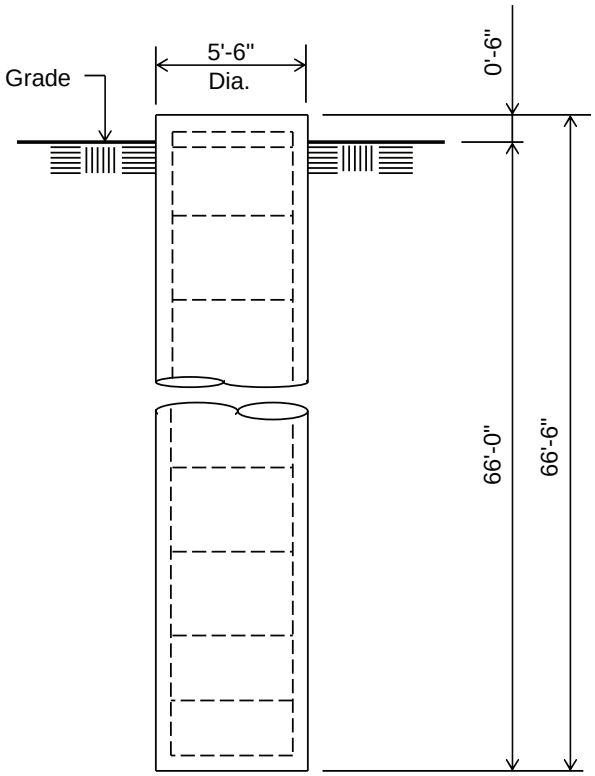
Rebar Schedule per Mat and per Pier	
Pier	(24) #7 vertical rebar w/ hooks at bottom w/ #4 rebar ties, two (2) within top 5" of pier then 4" C/C
Mat	(63) #10 horizontal rebar evenly spaced each way top and bottom. (252 total)
Anchor Bolts per Leg	
(6) 1.25" dia. x 63" F1554-105 on a 12.75" B.C. w/ 8" max. projection above concrete.	

7101 Southbridge Dr - P.O. Box 658 - Sioux City, IA 51102-0658 - Phone 712.258.6690 - Fax 712.258.8250

No.: 26-3777-TLJ
Date: 02/04/2026
By: ARH

Customer: MILESTONE COMMUNICATIONS, INC.
Site: Stockton Lake, VA
255 ft. Model S3TL Series HD2 Self Supporting Tower

PRELIMINARY -NOT FOR CONSTRUCTION-



ELEVATION VIEW
(58.5 cu. yds.)
(3 REQUIRED; NOT TO SCALE)

Notes:

1) Concrete shall have a minimum 28-day compressive strength of 4,500 psi, in accordance with ACI 318-14.

2) Rebar to conform to ASTM specification A615 Grade 60.

3) All rebar to have a minimum of 3" concrete cover.

4) All exposed concrete corners to be chamfered 3/4".

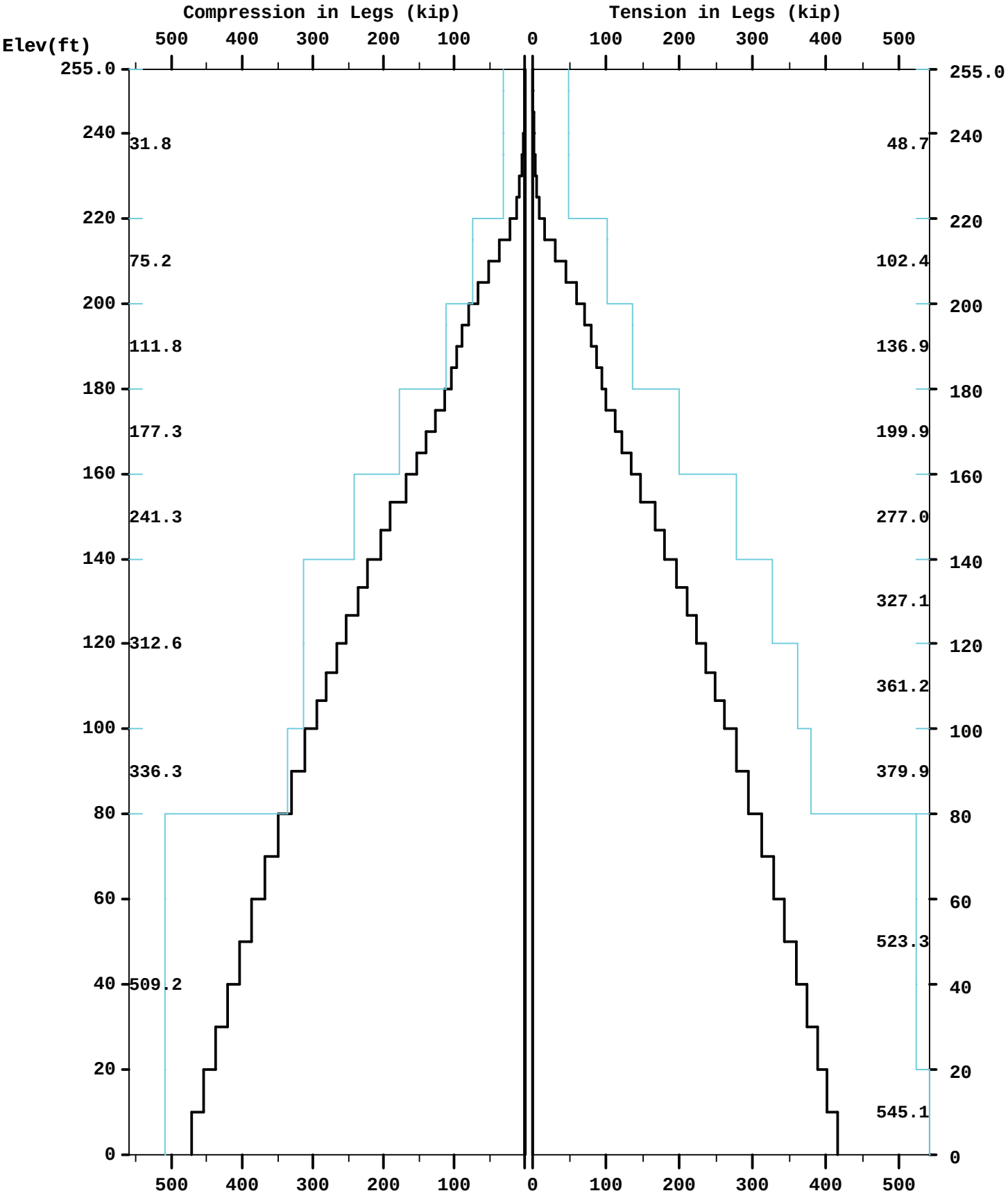
5) The foundation design is based on presumptive clay soil as defined in ANSI/TIA-222-H-2017. It is recommended that a soil analysis of the site be performed to verify the soil parameters used in the design.

6) The bottom anchor bolt template shall be positioned as closely as possible to the bottom of the anchor bolts.

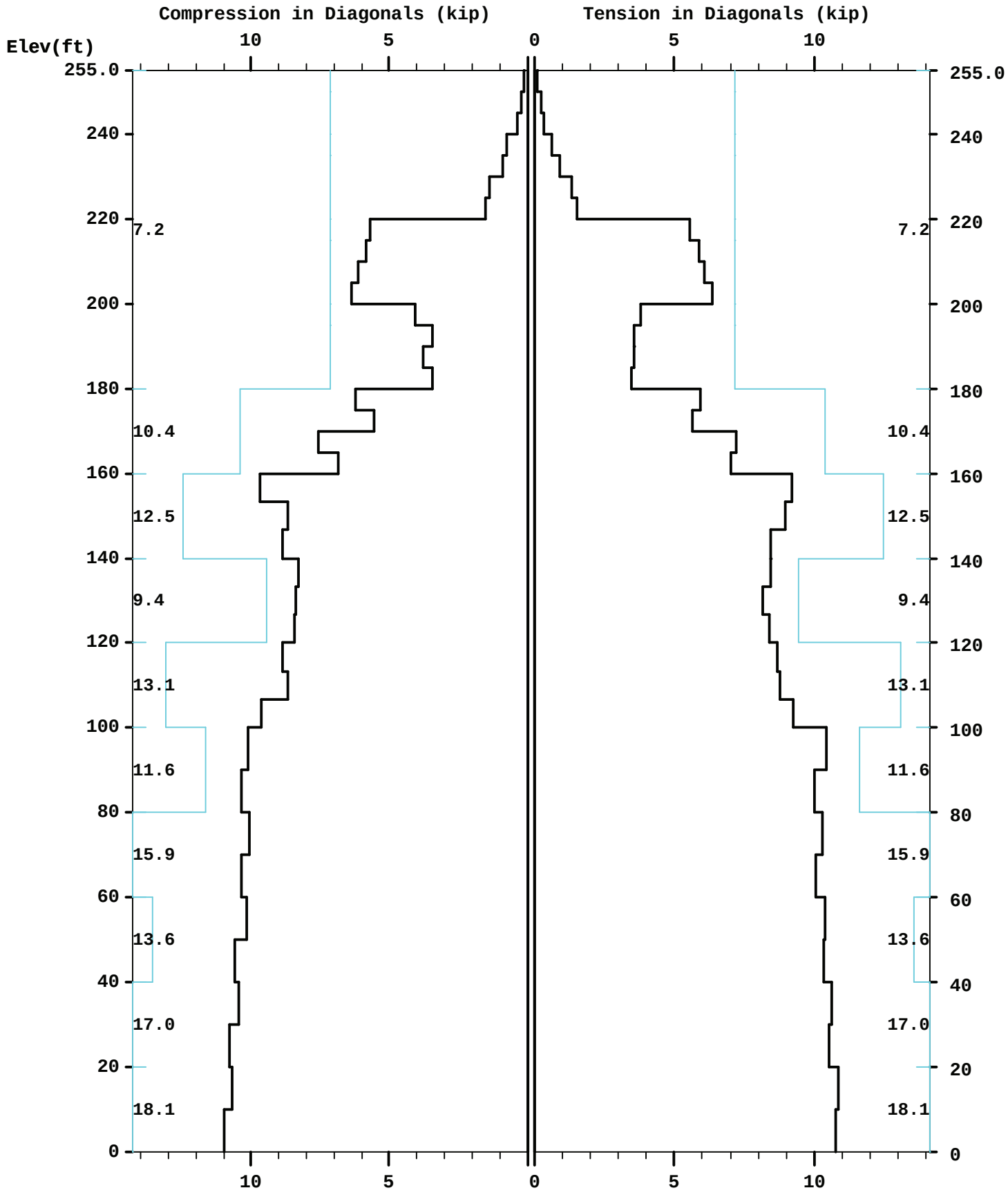
Rebar Schedule per Pier	
Pier	(18) #9 vertical rebar w/ #4 ties, two (2) within top 5" of pier then 12" C/C
Anchor Bolts per Leg	
(6) 1.25" dia. x 63" F1554-105 on a 12.75" B.C. w/ 8" max. projection above concrete.	

7101 Southbridge Dr - P.O. Box 658 - Sioux City, IA 51102-0658 - Phone 712.258.6690 - Fax 712.258.8250

Maximum

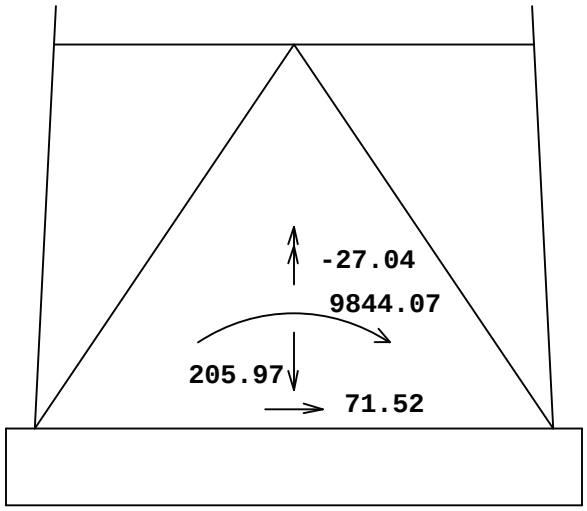

Compression in Legs (kip)
Tension in Legs (kip)
Elev(ft)
500 400 300 200 100 0 100 200 300 400 500
255.0
240
220
200
180
160
140
120
100
80
60
40
20
0
31.8
75.2
111.8
177.3
241.3
312.6
336.3
509.2
48.7
102.4
136.9
199.9
277.0
327.1
361.2
379.9
523.3
545.1

Maximum

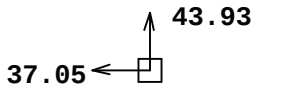

Compression in Diagonals (kip)
Tension in Diagonals (kip)
Elev(ft)
10
5
0
5
10
255.0
240
220
200
180
160
140
120
100
80
60
40
20
0
7.2
10.4
12.5
9.4
13.1
11.6
15.9
13.6
17.0
18.1

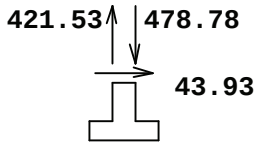
Maximum

TOTAL FOUNDATION LOADS (kip, ft-kip)



INDIVIDUAL FOOTING LOADS (kip)





MAST GEOMETRY (ft)

PANEL TYPE	NO.OF LEGS	ELEV.AT BOTTOM	ELEV.AT TOP	F.W..AT BOTTOM	F.W..AT TOP	TYPICAL PANEL HEIGHT
X	3	250.00	255.00	5.00	5.00	5.00
X	3	240.00	250.00	5.00	5.00	5.00
X	3	235.00	240.00	5.00	5.00	5.00
X	3	220.00	235.00	5.00	5.00	5.00
X	3	215.00	220.00	5.00	5.00	5.00
X	3	200.00	215.00	5.00	5.00	5.00
X	3	195.00	200.00	5.50	5.00	5.00
X	3	180.00	195.00	7.00	5.50	5.00
X	3	160.00	180.00	9.00	7.00	5.00
X	3	140.00	160.00	11.00	9.00	6.67
X	3	120.00	140.00	13.00	11.00	6.67
X	3	100.00	120.00	15.00	13.00	6.67
X	3	80.00	100.00	17.00	15.00	10.00
X	3	60.00	80.00	19.00	17.00	10.00
X	3	40.00	60.00	21.00	19.00	10.00
X	3	20.00	40.00	23.00	21.00	10.00
X	3	0.00	20.00	25.00	23.00	10.00

MEMBER PROPERTIES

MEMBER TYPE	BOTTOM ELEV ft	TOP ELEV ft	X-SECTN AREA in.sq	RADIUS OF GYRAT in	ELASTIC MODULUS ksi	THERMAL EXPANSN /deg
LE	220.00	255.00	1.075	0.787	29000.	0.0000117
LE	200.00	220.00	2.254	0.787	29000.	0.0000117
LE	180.00	200.00	3.016	0.787	29000.	0.0000117
LE	160.00	180.00	4.407	0.787	29000.	0.0000117
LE	140.00	160.00	6.111	0.787	29000.	0.0000117
LE	100.00	140.00	7.952	0.787	29000.	0.0000117
LE	80.00	100.00	8.399	0.787	29000.	0.0000117
LE	0.00	80.00	12.763	0.787	29000.	0.0000117
DI	180.00	255.00	0.484	0.626	29000.	0.0000117
DI	160.00	180.00	0.715	0.626	29000.	0.0000117
DI	120.00	160.00	0.902	0.626	29000.	0.0000117
DI	100.00	120.00	1.090	0.626	29000.	0.0000117
DI	80.00	100.00	1.438	0.626	29000.	0.0000117
DI	40.00	80.00	1.688	0.626	29000.	0.0000117
DI	0.00	40.00	1.938	0.626	29000.	0.0000117
HO	250.00	255.00	0.484	0.626	29000.	0.0000117
HO	235.00	240.00	0.484	0.626	29000.	0.0000117
HO	215.00	220.00	0.484	0.626	29000.	0.0000117

HO 195.00 200.00 0.484 0.626 29000. 0.0000117

FACTORED MEMBER RESISTANCES
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BOTTOM ELEV ft	TOP ELEV ft	LEGS COMP kip	LEGS TENS kip	DIAGONALS COMP kip	DIAGONALS TENS kip	HORIZONTALS COMP kip	HORIZONTALS TENS kip	INT BRACING COMP kip	INT BRACING TENS kip
250.0	255.0	31.84	48.70	7.16	7.16	7.16	7.16	0.00	0.00
240.0	250.0	31.84	48.70	7.16	7.16	0.00	0.00	0.00	0.00
235.0	240.0	31.84	48.70	7.16	7.16	7.16	7.16	0.00	0.00
220.0	235.0	31.84	48.70	7.16	7.16	0.00	0.00	0.00	0.00
215.0	220.0	75.23	102.40	7.16	7.16	7.16	7.16	0.00	0.00
200.0	215.0	75.23	102.40	7.16	7.16	0.00	0.00	0.00	0.00
195.0	200.0	111.82	136.93	7.16	7.16	7.16	7.16	0.00	0.00
180.0	195.0	111.82	136.93	7.16	7.16	0.00	0.00	0.00	0.00
160.0	180.0	177.29	199.92	10.38	10.38	0.00	0.00	0.00	0.00
140.0	160.0	241.28	277.05	12.47	12.47	0.00	0.00	0.00	0.00
120.0	140.0	312.59	327.10	9.45	9.45	0.00	0.00	0.00	0.00
100.0	120.0	312.59	361.16	13.10	13.10	0.00	0.00	0.00	0.00
80.0	100.0	336.31	379.88	11.64	11.64	0.00	0.00	0.00	0.00
60.0	80.0	509.22	523.32	15.88	15.88	0.00	0.00	0.00	0.00
40.0	60.0	509.22	523.32	13.59	13.59	0.00	0.00	0.00	0.00
20.0	40.0	509.22	523.32	17.02	17.02	0.00	0.00	0.00	0.00
0.0	20.0	509.22	545.12	18.13	18.13	0.00	0.00	0.00	0.00

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* Only 5 condition(s) shown in full
* Some wind loads may have been derived from full-scale wind tunnel testing

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LOADING CONDITION A ==========

110 mph wind with no ice. Wind Azimuth: 0° (1.2 D + 1.0 Wo) PL - 0

MAST LOADING
==========

LOAD TYPE	ELEV ft	APPLY..LOAD..AT RADIUS ft	AZI	LOAD AZI	.....FORCES..... HORIZ kip	DOWN kip	.....MOMENTS..... VERTICAL ft-kip	TORSNAL ft-kip
C	238.9	0.00	0.0	0.0	0.42	0.11	0.00	0.00
C	230.6	0.00	0.0	0.0	0.75	0.72	0.00	0.00
C	220.0	0.00	0.0	0.0	8.19	5.40	0.00	0.00
C	198.9	0.00	0.0	0.0	0.40	0.11	0.00	0.00
C	190.6	0.00	0.0	0.0	0.72	0.72	0.00	0.00
C	180.0	0.00	0.0	0.0	6.28	5.40	0.00	0.00
C	170.0	0.00	0.0	0.0	4.65	5.40	0.00	0.00
C	160.0	0.00	0.0	0.0	4.59	5.40	0.00	0.00
C	122.0	0.00	0.0	0.0	0.17	0.72	0.00	0.00
C	50.0	0.00	0.0	0.0	0.28	0.36	0.00	0.00
D	255.0	0.00	180.0	0.0	0.06	0.03	0.00	0.00
D	240.0	0.00	180.0	0.0	0.05	0.03	0.00	0.00
D	240.0	0.00	180.0	0.0	0.06	0.04	0.00	0.00

D	230.0	0.00	312.7	0.0	0.05	0.03	0.00	0.00
D	230.0	0.00	312.7	0.0	0.07	0.04	0.01	-0.03
D	220.0	0.00	312.7	0.0	0.07	0.04	0.01	-0.03
D	220.0	0.00	312.7	0.0	0.10	0.06	0.03	-0.06
D	200.0	0.00	312.7	0.0	0.09	0.06	0.03	-0.06
D	200.0	0.00	316.1	0.0	0.10	0.07	0.03	-0.06
D	190.0	0.00	318.6	0.0	0.10	0.07	0.03	-0.06
D	190.0	0.00	349.5	0.0	0.11	0.08	0.03	-0.02
D	180.0	0.00	347.7	0.0	0.12	0.08	0.03	-0.02
D	180.0	0.00	25.3	0.0	0.13	0.11	0.04	0.00
D	170.0	0.00	23.8	0.0	0.13	0.11	0.04	0.00
D	170.0	0.00	120.2	0.0	0.15	0.13	0.02	-0.01
D	160.0	0.00	120.7	0.0	0.15	0.13	0.02	-0.01
D	160.0	0.00	335.8	0.0	0.18	0.16	0.01	-0.05
D	140.0	0.00	329.5	0.0	0.19	0.17	0.01	-0.05
D	140.0	0.00	340.2	0.0	0.19	0.19	0.02	-0.05
D	120.0	0.00	338.5	0.0	0.19	0.20	0.02	-0.05
D	120.0	0.00	341.4	0.0	0.20	0.21	0.02	-0.05
D	100.0	0.00	341.4	0.0	0.21	0.22	0.03	-0.06
D	100.0	0.00	343.5	0.0	0.20	0.22	0.04	-0.05
D	80.0	0.00	342.6	0.0	0.20	0.22	0.04	-0.06
D	80.0	0.00	344.7	0.0	0.20	0.29	0.04	-0.05
D	40.0	0.00	345.7	0.0	0.20	0.31	0.05	-0.05
D	40.0	0.00	346.8	0.0	0.19	0.32	0.06	-0.05
D	20.0	0.00	346.4	0.0	0.19	0.33	0.06	-0.05
D	20.0	0.00	347.5	0.0	0.17	0.33	0.07	-0.04
D	0.0	0.00	347.1	0.0	0.18	0.34	0.07	-0.04

ANTENNA LOADING
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.....ANTENNA.....			ATTACHMENT		..........ANTENNA FORCES.....			
TYPE	ELEV	AZI	RAD	AZI	AXIAL	SHEAR	GRAVITY	TORSION
	ft		ft		kip	kip	kip	ft-kip
HP	122.0	0.0	8.9	0.0	1.11	0.00	0.34	0.00
HP	106.0	0.0	9.8	0.0	1.08	0.00	0.34	0.00

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LOADING CONDITION k ==========

110 mph wind with no ice. Wind Azimuth: 0° (0.9 D + 1.0 Wo) PL - 0

MAST LOADING
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LOAD	ELEV	APPLY..LOAD..AT		LOAD	......FORCES.....		......MOMENTS.....	
TYPE		RADIUS	AZI	AZI	HORIZ	DOWN	VERTICAL	TORSNAL
	ft	ft			kip	kip	ft-kip	ft-kip
C	238.9	0.00	0.0	0.0	0.42	0.08	0.00	0.00
C	230.6	0.00	0.0	0.0	0.75	0.54	0.00	0.00
C	220.0	0.00	0.0	0.0	8.19	4.05	0.00	0.00
C	198.9	0.00	0.0	0.0	0.40	0.08	0.00	0.00
C	190.6	0.00	0.0	0.0	0.72	0.54	0.00	0.00
C	180.0	0.00	0.0	0.0	6.28	4.05	0.00	0.00
C	170.0	0.00	0.0	0.0	4.65	4.05	0.00	0.00
C	160.0	0.00	0.0	0.0	4.59	4.05	0.00	0.00
C	122.0	0.00	0.0	0.0	0.17	0.54	0.00	0.00

C	50.0	0.00	0.0	0.0	0.28	0.27	0.00	0.00
D	255.0	0.00	180.0	0.0	0.06	0.03	0.00	0.00
D	240.0	0.00	180.0	0.0	0.05	0.02	0.00	0.00
D	240.0	0.00	180.0	0.0	0.06	0.03	0.00	0.00
D	230.0	0.00	312.7	0.0	0.05	0.02	0.00	0.00
D	230.0	0.00	312.7	0.0	0.07	0.03	0.01	-0.03
D	220.0	0.00	312.7	0.0	0.07	0.03	0.01	-0.03
D	220.0	0.00	312.7	0.0	0.10	0.05	0.02	-0.06
D	200.0	0.00	312.7	0.0	0.09	0.04	0.02	-0.06
D	200.0	0.00	316.1	0.0	0.10	0.06	0.02	-0.06
D	190.0	0.00	318.6	0.0	0.10	0.05	0.02	-0.06
D	190.0	0.00	349.5	0.0	0.11	0.06	0.02	-0.02
D	180.0	0.00	347.7	0.0	0.12	0.06	0.02	-0.02
D	180.0	0.00	25.3	0.0	0.13	0.08	0.03	0.00
D	170.0	0.00	23.8	0.0	0.13	0.08	0.03	0.00
D	170.0	0.00	120.2	0.0	0.15	0.09	0.01	-0.01
D	160.0	0.00	120.7	0.0	0.15	0.10	0.01	-0.01
D	160.0	0.00	335.8	0.0	0.18	0.12	0.01	-0.05
D	140.0	0.00	329.5	0.0	0.19	0.13	0.01	-0.05
D	140.0	0.00	340.2	0.0	0.19	0.14	0.01	-0.05
D	120.0	0.00	338.5	0.0	0.19	0.15	0.01	-0.05
D	120.0	0.00	341.4	0.0	0.20	0.16	0.02	-0.05
D	100.0	0.00	341.4	0.0	0.21	0.16	0.02	-0.06
D	100.0	0.00	343.5	0.0	0.20	0.17	0.03	-0.05
D	80.0	0.00	342.6	0.0	0.20	0.17	0.03	-0.06
D	80.0	0.00	344.7	0.0	0.20	0.22	0.03	-0.05
D	40.0	0.00	345.7	0.0	0.20	0.23	0.04	-0.05
D	40.0	0.00	346.8	0.0	0.19	0.24	0.05	-0.05
D	20.0	0.00	346.4	0.0	0.19	0.25	0.05	-0.05
D	20.0	0.00	347.5	0.0	0.17	0.25	0.05	-0.04
D	0.0	0.00	347.1	0.0	0.18	0.25	0.05	-0.04

ANTENNA LOADING
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.....ANTENNA.....			ATTACHMENT		..........ANTENNA FORCES.....			
TYPE	ELEV	AZI	RAD	AZI	AXIAL	SHEAR	GRAVITY	TORSION
	ft		ft		kip	kip	kip	ft-kip
HP	122.0	0.0	8.9	0.0	1.11	0.00	0.25	0.00
HP	106.0	0.0	9.8	0.0	1.08	0.00	0.25	0.00

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LOADING CONDITION AU ==========

30 mph wind with 1.5 ice. Wind Azimuth: 0° (1.2 D + 1.0 Di + 1.0 Wi) PL - 0

MAST LOADING
==========

LOAD	ELEV	APPLY..LOAD..AT		LOAD	......FORCES.....		......MOMENTS.....	
TYPE		RADIUS	AZI	AZI	HORIZ	DOWN	VERTICAL	TORSNAL
	ft	ft			kip	kip	ft-kip	ft-kip
C	238.9	0.00	0.0	0.0	0.31	0.29	0.00	0.00
C	230.6	0.00	0.0	0.0	0.12	1.56	0.00	0.00
C	220.0	0.00	0.0	0.0	1.12	14.78	0.00	0.00
C	198.9	0.00	0.0	0.0	0.29	0.29	0.00	0.00

C	190.6	0.00	0.0	0.0	0.11	1.54	0.00	0.00
C	180.0	0.00	0.0	0.0	0.85	14.60	0.00	0.00
C	170.0	0.00	0.0	0.0	0.63	14.55	0.00	0.00
C	160.0	0.00	0.0	0.0	0.62	14.49	0.00	0.00
C	122.0	0.00	0.0	0.0	0.02	1.11	0.00	0.00
C	50.0	0.00	0.0	0.0	0.04	0.73	0.00	0.00
D	255.0	0.00	180.0	0.0	0.01	0.21	0.00	0.00
D	250.0	0.00	180.0	0.0	0.01	0.21	0.00	0.00
D	250.0	0.00	180.0	0.0	0.01	0.17	0.00	0.00
D	240.0	0.00	180.0	0.0	0.01	0.17	0.00	0.00
D	240.0	0.00	180.0	0.0	0.01	0.21	0.00	0.00
D	235.0	0.00	180.0	0.0	0.01	0.21	0.00	0.00
D	235.0	0.00	312.7	0.0	0.01	0.17	0.01	0.00
D	230.0	0.00	312.7	0.0	0.01	0.17	0.01	0.00
D	230.0	0.00	312.7	0.0	0.01	0.20	0.08	-0.01
D	220.0	0.00	312.7	0.0	0.01	0.20	0.08	-0.01
D	220.0	0.00	312.7	0.0	0.01	0.30	0.17	-0.01
D	215.0	0.00	312.7	0.0	0.01	0.30	0.17	-0.01
D	215.0	0.00	312.7	0.0	0.01	0.26	0.17	-0.01
D	200.0	0.00	312.7	0.0	0.01	0.26	0.17	-0.01
D	200.0	0.00	316.1	0.0	0.01	0.31	0.18	-0.01
D	195.0	0.00	316.1	0.0	0.01	0.31	0.18	-0.01
D	195.0	0.00	318.9	0.0	0.01	0.28	0.17	-0.01
D	190.0	0.00	318.9	0.0	0.01	0.28	0.17	-0.01
D	190.0	0.00	352.1	0.0	0.01	0.32	0.18	0.00
D	180.0	0.00	350.3	0.0	0.01	0.32	0.17	0.00
D	180.0	0.00	25.3	0.0	0.02	0.40	0.24	0.00
D	170.0	0.00	23.8	0.0	0.02	0.41	0.24	0.00
D	170.0	0.00	113.8	0.0	0.02	0.48	0.09	0.00
D	160.0	0.00	114.4	0.0	0.02	0.49	0.08	0.00
D	160.0	0.00	345.6	0.0	0.02	0.58	0.08	-0.01
D	140.0	0.00	342.1	0.0	0.02	0.59	0.06	-0.01
D	140.0	0.00	348.4	0.0	0.02	0.62	0.10	-0.01
D	120.0	0.00	346.5	0.0	0.02	0.64	0.10	-0.01
D	120.0	0.00	347.4	0.0	0.02	0.68	0.15	-0.01
D	113.3	0.00	347.4	0.0	0.02	0.68	0.15	-0.01
D	113.3	0.00	346.7	0.0	0.02	0.69	0.14	-0.01
D	106.7	0.00	346.7	0.0	0.02	0.69	0.14	-0.01
D	106.7	0.00	346.7	0.0	0.02	0.70	0.18	-0.01
D	100.0	0.00	346.7	0.0	0.02	0.70	0.18	-0.01
D	100.0	0.00	348.1	0.0	0.02	0.67	0.21	-0.01
D	90.0	0.00	348.1	0.0	0.02	0.67	0.21	-0.01
D	90.0	0.00	347.4	0.0	0.02	0.68	0.20	-0.01
D	80.0	0.00	347.4	0.0	0.02	0.68	0.20	-0.01
D	80.0	0.00	351.8	0.0	0.02	0.77	0.25	-0.01
D	70.0	0.00	351.8	0.0	0.02	0.77	0.25	-0.01
D	70.0	0.00	349.6	0.0	0.02	0.77	0.23	-0.01
D	60.0	0.00	349.6	0.0	0.02	0.77	0.23	-0.01
D	60.0	0.00	353.0	0.0	0.02	0.78	0.27	-0.01
D	50.0	0.00	353.0	0.0	0.02	0.78	0.27	-0.01
D	50.0	0.00	352.4	0.0	0.02	0.78	0.31	-0.01
D	40.0	0.00	352.4	0.0	0.02	0.78	0.31	-0.01
D	40.0	0.00	352.7	0.0	0.02	0.81	0.33	-0.01
D	20.0	0.00	352.6	0.0	0.02	0.82	0.32	-0.01
D	20.0	0.00	351.1	0.0	0.02	0.65	0.14	0.00
D	10.0	0.00	351.1	0.0	0.02	0.65	0.14	0.00
D	10.0	0.00	352.6	0.0	0.02	0.73	0.27	0.00
D	0.0	0.00	352.6	0.0	0.02	0.73	0.27	0.00

ANTENNA LOADING

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.....ANTENNA.....			ATTACHMENT		.....ANTENNA FORCES.....			
TYPE	ELEV ft	AZI	RAD ft	AZI	AXIAL kip	SHEAR kip	GRAVITY kip	TORSION ft-kip
HP	122.0	0.0	8.9	0.0	0.09	0.00	1.20	0.00
HP	106.0	0.0	9.8	0.0	0.09	0.00	1.19	0.00

LOADING CONDITION CE ==========

Seismic - Azimuth: 0° (1.2 D + 1.0 Ev + 1.0 Eh) PL - 0

MAST LOADING
==========

LOAD	ELEV	APPLY..LOAD..AT		LOAD	......FORCES......		......MOMENTS......	
TYPE	ft	RADIUS ft	AZI	AZI	HORIZ kip	DOWN kip	VERTICAL ft-kip	TORSNAL ft-kip
C	247.5	0.00	0.0	0.0	0.05	0.70	0.00	0.00
C	238.9	0.00	0.0	0.0	0.01	0.11	0.00	0.00
C	230.6	0.00	0.0	0.0	0.05	0.74	0.00	0.00
C	230.0	0.00	0.0	0.0	0.06	0.89	0.00	0.00
C	225.3	0.00	0.0	0.0	0.00	0.06	0.00	0.00
C	220.0	0.00	0.0	0.0	0.33	5.58	0.00	0.00
C	210.0	0.00	0.0	0.0	0.01	0.27	0.00	0.00
C	210.0	0.00	0.0	0.0	0.07	1.22	0.00	0.00
C	198.9	0.00	0.0	0.0	0.01	0.11	0.00	0.00
C	195.3	0.00	0.0	0.0	0.01	0.13	0.00	0.00
C	190.6	0.00	0.0	0.0	0.04	0.74	0.00	0.00
C	190.0	0.00	0.0	0.0	0.07	1.52	0.00	0.00
C	185.3	0.00	0.0	0.0	0.01	0.15	0.00	0.00
C	185.3	0.00	0.0	0.0	0.00	0.06	0.00	0.00
C	180.0	0.00	0.0	0.0	0.25	5.58	0.00	0.00
C	175.0	0.00	0.0	0.0	0.01	0.14	0.00	0.00
C	175.0	0.00	0.0	0.0	0.01	0.14	0.00	0.00
C	170.0	0.00	0.0	0.0	0.23	5.58	0.00	0.00
C	170.0	0.00	0.0	0.0	0.09	2.19	0.00	0.00
C	165.0	0.00	0.0	0.0	0.01	0.14	0.00	0.00
C	165.0	0.00	0.0	0.0	0.01	0.14	0.00	0.00
C	165.0	0.00	0.0	0.0	0.01	0.13	0.00	0.00
C	160.0	0.00	0.0	0.0	0.21	5.58	0.00	0.00
C	150.0	0.00	0.0	0.0	0.01	0.20	0.00	0.00
C	150.0	0.00	0.0	0.0	0.01	0.27	0.00	0.00
C	150.0	0.00	0.0	0.0	0.10	2.74	0.00	0.00
C	150.0	0.00	0.0	0.0	0.01	0.27	0.00	0.00
C	150.0	0.00	0.0	0.0	0.01	0.31	0.00	0.00
C	131.0	0.00	0.0	0.0	0.01	0.25	0.00	0.00
C	131.0	0.00	0.0	0.0	0.01	0.18	0.00	0.00
C	131.0	0.00	0.0	0.0	0.01	0.25	0.00	0.00
C	131.0	0.00	0.0	0.0	0.01	0.27	0.00	0.00
C	130.0	0.00	0.0	0.0	0.10	3.34	0.00	0.00
C	122.0	0.00	0.0	0.0	0.02	0.74	0.00	0.00
C	122.0	0.00	0.0	0.0	0.01	0.24	0.00	0.00
C	122.0	0.00	0.0	0.0	0.00	0.06	0.00	0.00
C	122.0	0.00	0.0	0.0	0.01	0.30	0.00	0.00
C	121.0	0.00	0.0	0.0	0.00	0.03	0.00	0.00

C	121.0	0.00	0.0	0.0	0.00	0.03	0.00	0.00
C	121.0	0.00	0.0	0.0	0.00	0.02	0.00	0.00
C	121.0	0.00	0.0	0.0	0.00	0.03	0.00	0.00
C	113.0	0.00	0.0	0.0	0.00	0.19	0.00	0.00
C	113.0	0.00	0.0	0.0	0.00	0.19	0.00	0.00
C	113.0	0.00	0.0	0.0	0.00	0.16	0.00	0.00
C	113.0	0.00	0.0	0.0	0.01	0.21	0.00	0.00
C	110.0	0.00	0.0	0.0	0.09	3.90	0.00	0.00
C	106.0	0.00	0.0	0.0	0.01	0.30	0.00	0.00
C	106.0	0.00	0.0	0.0	0.00	0.06	0.00	0.00
C	106.0	0.00	0.0	0.0	0.01	0.24	0.00	0.00
C	103.0	0.00	0.0	0.0	0.00	0.09	0.00	0.00
C	103.0	0.00	0.0	0.0	0.00	0.08	0.00	0.00
C	103.0	0.00	0.0	0.0	0.00	0.08	0.00	0.00
C	103.0	0.00	0.0	0.0	0.00	0.08	0.00	0.00
C	90.0	0.00	0.0	0.0	0.00	0.25	0.00	0.00
C	90.0	0.00	0.0	0.0	0.00	0.27	0.00	0.00
C	90.0	0.00	0.0	0.0	0.00	0.27	0.00	0.00
C	90.0	0.00	0.0	0.0	0.01	0.31	0.00	0.00
C	90.0	0.00	0.0	0.0	0.07	4.06	0.00	0.00
C	70.0	0.00	0.0	0.0	0.00	0.27	0.00	0.00
C	70.0	0.00	0.0	0.0	0.00	0.31	0.00	0.00
C	70.0	0.00	0.0	0.0	0.00	0.25	0.00	0.00
C	70.0	0.00	0.0	0.0	0.00	0.27	0.00	0.00
C	70.0	0.00	0.0	0.0	0.07	5.60	0.00	0.00
C	55.0	0.00	0.0	0.0	0.00	0.15	0.00	0.00
C	55.0	0.00	0.0	0.0	0.00	0.13	0.00	0.00
C	55.0	0.00	0.0	0.0	0.00	0.14	0.00	0.00
C	55.0	0.00	0.0	0.0	0.00	0.14	0.00	0.00
C	50.0	0.00	0.0	0.0	0.00	0.37	0.00	0.00
C	50.0	0.00	0.0	0.0	0.05	5.77	0.00	0.00
C	45.0	0.00	0.0	0.0	0.00	0.13	0.00	0.00
C	45.0	0.00	0.0	0.0	0.00	0.15	0.00	0.00
C	45.0	0.00	0.0	0.0	0.00	0.14	0.00	0.00
C	45.0	0.00	0.0	0.0	0.00	0.14	0.00	0.00
C	30.0	0.00	0.0	0.0	0.00	0.27	0.00	0.00
C	30.0	0.00	0.0	0.0	0.00	0.27	0.00	0.00
C	30.0	0.00	0.0	0.0	0.00	0.31	0.00	0.00
C	30.0	0.00	0.0	0.0	0.00	0.27	0.00	0.00
C	30.0	0.00	0.0	0.0	0.03	6.21	0.00	0.00
C	10.0	0.00	0.0	0.0	0.00	0.31	0.00	0.00
C	10.0	0.00	0.0	0.0	0.00	0.27	0.00	0.00
C	10.0	0.00	0.0	0.0	0.00	0.27	0.00	0.00
C	10.0	0.00	0.0	0.0	0.00	0.27	0.00	0.00
C	10.0	0.00	0.0	0.0	0.01	6.51	0.00	0.00
D	255.0	0.00	180.0	180.0	0.00	0.00	0.00	0.00
D	0.0	0.00	180.0	180.0	0.00	0.00	0.00	0.00

ANTENNA LOADING
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.....ANTENNA.....			ATTACHMENT		.....ANTENNA FORCES.....			
TYPE	ELEV	AZI	RAD	AZI	AXIAL	SHEAR	GRAVITY	TORSION
	ft		ft		kip	kip	kip	ft-kip
HP	122.0	0.0	8.9	0.0	0.00	0.00	0.00	0.00
HP	106.0	0.0	9.8	0.0	0.00	0.00	0.00	0.00

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LOADING CONDITION CN ==========

Seismic - Azimuth: 0° (0.9 D - 1.0 Ev + 1.0 Eh) PL - 0

MAST LOADING
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LOAD TYPE	ELEV ft	APPLY..LOAD..AT RADIUS ft	AZI	LOAD AZI	......FORCES...... HORIZ kip	DOWN kip	......MOMENTS...... VERTICAL ft-kip	TORSNAL ft-kip
C	247.5	0.00	0.0	0.0	0.05	0.49	0.00	0.00
C	238.9	0.00	0.0	0.0	0.01	0.08	0.00	0.00
C	230.6	0.00	0.0	0.0	0.05	0.52	0.00	0.00
C	230.0	0.00	0.0	0.0	0.06	0.62	0.00	0.00
C	225.3	0.00	0.0	0.0	0.00	0.04	0.00	0.00
C	220.0	0.00	0.0	0.0	0.33	3.87	0.00	0.00
C	210.0	0.00	0.0	0.0	0.01	0.19	0.00	0.00
C	210.0	0.00	0.0	0.0	0.07	0.85	0.00	0.00
C	198.9	0.00	0.0	0.0	0.01	0.08	0.00	0.00
C	195.3	0.00	0.0	0.0	0.01	0.09	0.00	0.00
C	190.6	0.00	0.0	0.0	0.04	0.52	0.00	0.00
C	190.0	0.00	0.0	0.0	0.07	1.06	0.00	0.00
C	185.3	0.00	0.0	0.0	0.01	0.10	0.00	0.00
C	185.3	0.00	0.0	0.0	0.00	0.04	0.00	0.00
C	180.0	0.00	0.0	0.0	0.25	3.87	0.00	0.00
C	175.0	0.00	0.0	0.0	0.01	0.10	0.00	0.00
C	175.0	0.00	0.0	0.0	0.01	0.10	0.00	0.00
C	170.0	0.00	0.0	0.0	0.23	3.87	0.00	0.00
C	170.0	0.00	0.0	0.0	0.09	1.52	0.00	0.00
C	165.0	0.00	0.0	0.0	0.01	0.10	0.00	0.00
C	165.0	0.00	0.0	0.0	0.01	0.10	0.00	0.00
C	165.0	0.00	0.0	0.0	0.01	0.09	0.00	0.00
C	160.0	0.00	0.0	0.0	0.21	3.87	0.00	0.00
C	150.0	0.00	0.0	0.0	0.01	0.14	0.00	0.00
C	150.0	0.00	0.0	0.0	0.01	0.19	0.00	0.00
C	150.0	0.00	0.0	0.0	0.10	1.90	0.00	0.00
C	150.0	0.00	0.0	0.0	0.01	0.19	0.00	0.00
C	150.0	0.00	0.0	0.0	0.01	0.21	0.00	0.00
C	131.0	0.00	0.0	0.0	0.01	0.17	0.00	0.00
C	131.0	0.00	0.0	0.0	0.01	0.13	0.00	0.00
C	131.0	0.00	0.0	0.0	0.01	0.17	0.00	0.00
C	131.0	0.00	0.0	0.0	0.01	0.19	0.00	0.00
C	130.0	0.00	0.0	0.0	0.10	2.32	0.00	0.00
C	122.0	0.00	0.0	0.0	0.02	0.52	0.00	0.00
C	122.0	0.00	0.0	0.0	0.01	0.16	0.00	0.00
C	122.0	0.00	0.0	0.0	0.00	0.04	0.00	0.00
C	122.0	0.00	0.0	0.0	0.01	0.21	0.00	0.00
C	121.0	0.00	0.0	0.0	0.00	0.02	0.00	0.00
C	121.0	0.00	0.0	0.0	0.00	0.02	0.00	0.00
C	121.0	0.00	0.0	0.0	0.00	0.02	0.00	0.00
C	121.0	0.00	0.0	0.0	0.00	0.02	0.00	0.00
C	113.0	0.00	0.0	0.0	0.00	0.13	0.00	0.00
C	113.0	0.00	0.0	0.0	0.00	0.13	0.00	0.00
C	113.0	0.00	0.0	0.0	0.00	0.11	0.00	0.00
C	113.0	0.00	0.0	0.0	0.01	0.15	0.00	0.00
C	110.0	0.00	0.0	0.0	0.09	2.71	0.00	0.00
C	106.0	0.00	0.0	0.0	0.01	0.21	0.00	0.00
C	106.0	0.00	0.0	0.0	0.00	0.04	0.00	0.00
C	106.0	0.00	0.0	0.0	0.01	0.16	0.00	0.00

C	103.0	0.00	0.0	0.0	0.00	0.06	0.00	0.00
C	103.0	0.00	0.0	0.0	0.00	0.06	0.00	0.00
C	103.0	0.00	0.0	0.0	0.00	0.06	0.00	0.00
C	103.0	0.00	0.0	0.0	0.00	0.05	0.00	0.00
C	90.0	0.00	0.0	0.0	0.00	0.18	0.00	0.00
C	90.0	0.00	0.0	0.0	0.00	0.19	0.00	0.00
C	90.0	0.00	0.0	0.0	0.00	0.19	0.00	0.00
C	90.0	0.00	0.0	0.0	0.01	0.21	0.00	0.00
C	90.0	0.00	0.0	0.0	0.07	2.82	0.00	0.00
C	70.0	0.00	0.0	0.0	0.00	0.19	0.00	0.00
C	70.0	0.00	0.0	0.0	0.00	0.21	0.00	0.00
C	70.0	0.00	0.0	0.0	0.00	0.18	0.00	0.00
C	70.0	0.00	0.0	0.0	0.00	0.19	0.00	0.00
C	70.0	0.00	0.0	0.0	0.07	3.89	0.00	0.00
C	55.0	0.00	0.0	0.0	0.00	0.11	0.00	0.00
C	55.0	0.00	0.0	0.0	0.00	0.09	0.00	0.00
C	55.0	0.00	0.0	0.0	0.00	0.10	0.00	0.00
C	55.0	0.00	0.0	0.0	0.00	0.10	0.00	0.00
C	50.0	0.00	0.0	0.0	0.00	0.26	0.00	0.00
C	50.0	0.00	0.0	0.0	0.05	4.01	0.00	0.00
C	45.0	0.00	0.0	0.0	0.00	0.09	0.00	0.00
C	45.0	0.00	0.0	0.0	0.00	0.11	0.00	0.00
C	45.0	0.00	0.0	0.0	0.00	0.10	0.00	0.00
C	45.0	0.00	0.0	0.0	0.00	0.10	0.00	0.00
C	30.0	0.00	0.0	0.0	0.00	0.19	0.00	0.00
C	30.0	0.00	0.0	0.0	0.00	0.19	0.00	0.00
C	30.0	0.00	0.0	0.0	0.00	0.21	0.00	0.00
C	30.0	0.00	0.0	0.0	0.00	0.19	0.00	0.00
C	30.0	0.00	0.0	0.0	0.03	4.32	0.00	0.00
C	10.0	0.00	0.0	0.0	0.00	0.21	0.00	0.00
C	10.0	0.00	0.0	0.0	0.00	0.19	0.00	0.00
C	10.0	0.00	0.0	0.0	0.00	0.19	0.00	0.00
C	10.0	0.00	0.0	0.0	0.00	0.19	0.00	0.00
C	10.0	0.00	0.0	0.0	0.01	4.52	0.00	0.00
D	255.0	0.00	180.0	180.0	0.00	0.00	0.00	0.00
D	0.0	0.00	180.0	180.0	0.00	0.00	0.00	0.00

ANTENNA LOADING
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.....ANTENNA.....			ATTACHMENT		..........ANTENNA FORCES.....			
TYPE	ELEV	AZI	RAD	AZI	AXIAL	SHEAR	GRAVITY	TORSION
	ft		ft		kip	kip	kip	ft-kip
HP	122.0	0.0	8.9	0.0	0.00	0.00	0.00	0.00
HP	106.0	0.0	9.8	0.0	0.00	0.00	0.00	0.00

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MAXIMUM ANTENNA AND REFLECTOR ROTATIONS:
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ELEV	AZI	TYPE	......BEAM DEFLECTIONS (deg).....			
ft	deg	*	ROLL	YAW	PITCH	TOTAL
122.0	0.0	HP	-0.714 S	0.046 AN	-0.668 b	0.669 b
106.0	0.0	HP	-0.589 S	0.039 AN	-0.550 b	0.551 b

MAXIMUM TENSION IN MAST MEMBERS (kip)
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ELEV ft	LEGS	DIAG	HORIZ	BRACE
255.0	----------		0.02 BI	0.00 A
	0.04 AJ	0.08 AI		
250.0	----------		0.00 BA	0.00 A
	0.34 AI	0.22 J		
245.0	----------		0.01 BG	0.00 A
	0.96 k	0.35 AL		
240.0	----------		0.11 BO	0.00 A
	2.02 k	0.63 AL		
235.0	----------		0.08 A	0.00 A
	3.79 k	0.89 J		
230.0	----------		0.05 S	0.00 A
	6.23 k	1.34 AR		
225.0	----------		0.13 A	0.00 A
	9.58 k	1.51 h		
220.0	----------		1.14 U	0.00 A
	16.44 k	5.54 AR		
215.0	----------		0.26 A	0.00 A
	30.71 k	5.89 P		
210.0	----------		0.06 AC	0.00 A
	44.75 k	6.07 z		
205.0	----------		0.30 A	0.00 A
	59.60 k	6.34 P		
200.0	----------		0.26 AC	0.00 A
	70.85 k	3.79 k		
195.0	----------		0.31 A	0.00 A
	79.70 k	3.55 S		
190.0	----------		0.02 g	0.00 A
	86.37 k	3.55 k		
185.0	----------		0.31 A	0.00 A
	93.92 k	3.46 j		
180.0	----------		0.09 I	0.00 A
	100.68 k	5.91 k		
175.0	----------		0.30 A	0.00 A
	112.67 k	5.64 S		
170.0	----------		0.10 I	0.00 A
	121.33 k	7.23 k		
165.0	----------		0.31 A	0.00 A
	134.99 k	7.03 S		
160.0	----------		0.09 I	0.00 A
	146.61 k	9.20 k		
153.3	----------		0.27 A	0.00 A
	166.13 k	8.95 S		
146.7	----------		0.03 I	0.00 A
	180.43 k	8.44 AT		
140.0	----------		0.20 A	0.00 A
	196.80 k	8.45 j		
133.3	----------		0.02 L	0.00 A
	209.68 k	8.17 AB		
126.7	----------		0.15 A	0.00 A
	224.05 k	8.42 R		
120.0	----------		0.02 S	0.00 A
	236.07 k	8.68 AB		
113.3	----------		0.20 A	0.00 A
	249.43 k	8.78 j		
106.7	----------		0.03 S	0.00 A
	261.67 k	9.25 AB		
100.0	----------		0.13 M	0.00 A

	276.91 k	10.43 j		
90.0	----------		0.10 A	0.00 A
	294.79 k	10.02 AB		
80.0	----------		0.10 A	0.00 A
	311.65 k	10.30 j		
70.0	----------		0.05 A	0.00 A
	327.87 k	10.06 AB		
60.0	----------		0.09 A	0.00 A
	343.45 k	10.37 j		
50.0	----------		0.05 A	0.00 A
	358.58 k	10.33 AB		
40.0	----------		0.08 A	0.00 A
	373.41 k	10.64 j		
30.0	----------		0.05 A	0.00 A
	387.79 k	10.55 AB		
20.0	----------		0.01 b	0.00 A
	401.83 k	10.86 j		
10.0	----------		0.06 A	0.00 A
	415.43 k	10.77 AB		
0.0	----------		0.00 A	0.00 A

MAXIMUM COMPRESSION IN MAST MEMBERS (kip)
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ELEV ft	LEGS	DIAG	HORIZ	BRACE
255.0	----------		0.00 AE	0.00 A
	-0.16 BB	-0.11 S		
250.0	----------		0.00 k	0.00 A
	-0.58 S	-0.23 AC		
245.0	----------		-0.01 AC	0.00 A
	-1.38 G	-0.38 G		
240.0	----------		-0.03 AO	0.00 A
	-2.70 G	-0.73 G		
235.0	----------		-0.07 q	0.00 A
	-4.96 G	-0.87 AL		
230.0	----------		-0.05 k	0.00 A
	-8.06 G	-1.36 h		
225.0	----------		-0.12 q	0.00 A
	-11.84 G	-1.52 P		
220.0	----------		-1.05 m	0.00 A
	-22.14 S	-5.68 P		
215.0	----------		-0.22 AC	0.00 A
	-37.34 S	-5.82 AR		
210.0	----------		-0.06 A	0.00 A
	-51.83 S	-6.14 P		
205.0	----------		-0.26 AC	0.00 A
	-67.43 S	-6.35 P		
200.0	----------		-0.30 A	0.00 A
	-79.09 S	-4.06 S		
195.0	----------		-0.28 AC	0.00 A
	-88.81 S	-3.45 AB		
190.0	----------		-0.02 y	0.00 A
	-96.07 S	-3.76 S		
185.0	----------		-0.28 AC	0.00 A

```
          -104.26 S      -3.43 AB
   180.0  ---------------------         -0.09 AK      0.00 A
          -114.45 S      -6.20 S
   175.0  ---------------------         -0.27 AC      0.00 A
          -127.33 S      -5.53 AT
   170.0  ---------------------         -0.10 AK      0.00 A
          -139.40 S      -7.59 S
   165.0  ---------------------         -0.28 AC      0.00 A
          -154.09 S      -6.85 AT
   160.0  ---------------------         -0.09 AK      0.00 A
          -169.17 S      -9.66 S
   153.3  ---------------------         -0.23 AC      0.00 A
          -190.28 S      -8.69 AT
   146.7  ---------------------         -0.03 AK      0.00 A
          -205.13 S      -8.84 S
   140.0  ---------------------         -0.18 AC      0.00 A
          -222.97 S      -8.30 AB
   133.3  ---------------------         -0.01 AK      0.00 A
          -236.69 S      -8.39 S
   126.7  ---------------------         -0.13 AC      0.00 A
          -252.68 S      -8.45 AT
   120.0  ---------------------         -0.02 k       0.00 A
          -266.36 S      -8.86 j
   113.3  ---------------------         -0.17 AC      0.00 A
          -280.98 S      -8.67 AB
   106.7  ---------------------         -0.03 k       0.00 A
          -294.68 S      -9.62 j
   100.0  ---------------------         -0.11 AC      0.00 A
          -311.63 S     -10.12 AB
    90.0  ---------------------         -0.09 AO      0.00 A
          -330.92 S     -10.37 j
    80.0  ---------------------         -0.09 AC      0.00 A
          -350.08 S     -10.05 AB
    70.0  ---------------------         -0.05 AO      0.00 A
          -368.31 S     -10.36 j
    60.0  ---------------------         -0.08 AC      0.00 A
          -386.44 S     -10.16 AB
    50.0  ---------------------         -0.04 q       0.00 A
          -403.97 S     -10.60 j
    40.0  ---------------------         -0.07 AC      0.00 A
          -421.47 S     -10.47 AB
    30.0  ---------------------         -0.04 AC      0.00 A
          -438.30 S     -10.79 j
    20.0  ---------------------         -0.01 t       0.00 A
          -455.14 S     -10.70 AB
    10.0  ---------------------         -0.05 AC      0.00 A
          -471.30 S     -11.00 j
     0.0  ---------------------          0.00 A       0.00 A


FORCE/RESISTANCE RATIO IN LEGS
==============================

          -- LEG COMPRESSION -    ---- LEG TENSION ---
  MAST                   FORCE/                   FORCE/
  ELEV     MAX  COMP    RESIST    MAX   TENS    RESIST
    ft    COMP  RESIST  RATIO     TENS  RESIST  RATIO

255.00 ------------------------------------------------
          0.16   31.84    0.00    0.04   48.70    0.00
250.00 ------------------------------------------------
```

```
          0.58   31.84    0.02    0.34   48.70    0.01
245.00 ----------------------------------------------
          1.38   31.84    0.04    0.96   48.70    0.02
240.00 ----------------------------------------------
          2.70   31.84    0.08    2.02   48.70    0.04
235.00 ----------------------------------------------
          4.96   31.84    0.16    3.79   48.70    0.08
230.00 ----------------------------------------------
          8.06   31.84    0.25    6.23   48.70    0.13
225.00 ----------------------------------------------
         11.84   31.84    0.37    9.58   48.70    0.20
220.00 ----------------------------------------------
         22.14   75.23    0.29   16.44  102.40    0.16
215.00 ----------------------------------------------
         37.34   75.23    0.50   30.71  102.40    0.30
210.00 ----------------------------------------------
         51.83   75.23    0.69   44.75  102.40    0.44
205.00 ----------------------------------------------
         67.43   75.23    0.90   59.60  102.40    0.58
200.00 ----------------------------------------------
         79.09  111.82    0.71   70.85  136.93    0.52
195.00 ----------------------------------------------
         88.81  111.82    0.79   79.70  136.93    0.58
190.00 ----------------------------------------------
         96.07  111.82    0.86   86.37  136.93    0.63
185.00 ----------------------------------------------
        104.26  111.82    0.93   93.92  136.93    0.69
180.00 ----------------------------------------------
        114.45  177.29    0.65  100.68  199.92    0.50
175.00 ----------------------------------------------
        127.33  177.29    0.72  112.67  199.92    0.56
170.00 ----------------------------------------------
        139.40  177.29    0.79  121.33  199.92    0.61
165.00 ----------------------------------------------
        154.09  177.29    0.87  134.99  199.92    0.68
160.00 ----------------------------------------------
        169.17  241.28    0.70  146.61  277.05    0.53
153.33 ----------------------------------------------
        190.28  241.28    0.79  166.13  277.05    0.60
146.67 ----------------------------------------------
        205.13  241.28    0.85  180.43  277.05    0.65
140.00 ----------------------------------------------
        222.97  312.59    0.71  196.80  327.10    0.60
133.33 ----------------------------------------------
        236.69  312.59    0.76  209.68  327.10    0.64
126.67 ----------------------------------------------
        252.68  312.59    0.81  224.05  327.10    0.68
120.00 ----------------------------------------------
        266.36  312.59    0.85  236.07  361.16    0.65
113.33 ----------------------------------------------
        280.98  312.59    0.90  249.43  361.16    0.69
106.67 ----------------------------------------------
        294.68  312.59    0.94  261.67  361.16    0.72
100.00 ----------------------------------------------
        311.63  336.31    0.93  276.91  379.88    0.73
 90.00 ----------------------------------------------
        330.92  336.31    0.98  294.79  379.88    0.78
 80.00 ----------------------------------------------
        350.08  509.22    0.69  311.65  523.32    0.60
 70.00 ----------------------------------------------
        368.31  509.22    0.72  327.87  523.32    0.63
```

60.00						
	386.44	509.22	0.76	343.45	523.32	0.66
50.00						
	403.97	509.22	0.79	358.58	523.32	0.69
40.00						
	421.47	509.22	0.83	373.41	523.32	0.71
30.00						
	438.30	509.22	0.86	387.79	523.32	0.74
20.00						
	455.14	509.22	0.89	401.83	545.12	0.74
10.00						
	471.30	509.22	0.93	415.43	545.12	0.76
0.00						

FORCE/RESISTANCE RATIO IN DIAGONALS
==========

MAST ELEV ft	- DIAG COMPRESSION - MAX COMP	COMP RESIST	FORCE/ RESIST RATIO	--- DIAG TENSION -- MAX TENS	TENS RESIST	FORCE/ RESIST RATIO
255.00						
	0.11	7.16	0.01	0.08	7.16	0.01
250.00						
	0.23	7.16	0.03	0.22	7.16	0.03
245.00						
	0.38	7.16	0.05	0.35	7.16	0.05
240.00						
	0.73	7.16	0.10	0.63	7.16	0.09
235.00						
	0.87	7.16	0.12	0.89	7.16	0.12
230.00						
	1.36	7.16	0.19	1.34	7.16	0.19
225.00						
	1.52	7.16	0.21	1.51	7.16	0.21
220.00						
	5.68	7.16	0.79	5.54	7.16	0.77
215.00						
	5.82	7.16	0.81	5.89	7.16	0.82
210.00						
	6.14	7.16	0.86	6.07	7.16	0.85
205.00						
	6.35	7.16	0.89	6.34	7.16	0.89
200.00						
	4.06	7.16	0.57	3.79	7.16	0.53
195.00						
	3.45	7.16	0.48	3.55	7.16	0.50
190.00						
	3.76	7.16	0.53	3.55	7.16	0.50
185.00						
	3.43	7.16	0.48	3.46	7.16	0.48
180.00						
	6.20	10.38	0.60	5.91	10.38	0.57
175.00						
	5.53	10.38	0.53	5.64	10.38	0.54
170.00						
	7.59	10.38	0.73	7.23	10.38	0.70
165.00						
	6.85	10.38	0.66	7.03	10.38	0.68
160.00						

```
          9.66   12.47    0.77    9.20   12.47    0.74
153.33 ---------------------------------------------
          8.69   12.47    0.70    8.95   12.47    0.72
146.67 ---------------------------------------------
          8.84   12.47    0.71    8.44   12.47    0.68
140.00 ---------------------------------------------
          8.30    9.45    0.88    8.45    9.45    0.89
133.33 ---------------------------------------------
          8.39    9.45    0.89    8.17    9.45    0.86
126.67 ---------------------------------------------
          8.45    9.45    0.89    8.42    9.45    0.89
120.00 ---------------------------------------------
          8.86   13.10    0.68    8.68   13.10    0.66
113.33 ---------------------------------------------
          8.67   13.10    0.66    8.78   13.10    0.67
106.67 ---------------------------------------------
          9.62   13.10    0.73    9.25   13.10    0.71
100.00 ---------------------------------------------
         10.12   11.64    0.87   10.43   11.64    0.90
 90.00 ---------------------------------------------
         10.37   11.64    0.89   10.02   11.64    0.86
 80.00 ---------------------------------------------
         10.05   15.88    0.63   10.30   15.88    0.65
 70.00 ---------------------------------------------
         10.36   15.88    0.65   10.06   15.88    0.63
 60.00 ---------------------------------------------
         10.16   13.59    0.75   10.37   13.59    0.76
 50.00 ---------------------------------------------
         10.60   13.59    0.78   10.33   13.59    0.76
 40.00 ---------------------------------------------
         10.47   17.02    0.62   10.64   17.02    0.63
 30.00 ---------------------------------------------
         10.79   17.02    0.63   10.55   17.02    0.62
 20.00 ---------------------------------------------
         10.70   18.13    0.59   10.86   18.13    0.60
 10.00 ---------------------------------------------
         11.00   18.13    0.61   10.77   18.13    0.59
  0.00 ---------------------------------------------

MAXIMUM INDIVIDUAL FOUNDATION LOADS: (kip)
==========================================

    --------------LOAD---COMPONENTS-------------        TOTAL
    NORTH          EAST          DOWN       UPLIFT        SHEAR

    43.93 S     -37.05 G     478.78 S    -421.53 k      43.93 S


MAXIMUM TOTAL LOADS ON FOUNDATION : (kip & kip-ft)
==================================================

  ------HORIZONTAL-----    DOWN       ---------OVERTURNING--------- TORSION
  NORTH    EAST   TOTAL               NORTH        EAST       TOTAL
               @   0.0                                      @   0.0

   71.5   -65.5    71.5   206.0      9844.1      9135.4      9844.1   -27.0
     S       t       S       BQ          S           b           S       n


================================================================================
```

****
********** Service Load Condition **********
****

* Only 1 condition(s) shown in full
* Some wind loads may have been derived from full-scale wind tunnel testing

LOADING CONDITION A

60 mph wind with no ice. Wind Azimuth: 0° (1.0 D + 1.0 Wo) PL - 0

MAST LOADING

LOAD TYPE	ELEV ft	APPLY..LOAD..AT RADIUS ft	AZI	LOAD AZI	.....FORCES..... HORIZ kip	DOWN kip	.....MOMENTS..... VERTICAL ft-kip	TORSNAL ft-kip
C	238.9	0.00	0.0	0.0	0.12	0.09	0.00	0.00
C	230.6	0.00	0.0	0.0	0.22	0.60	0.00	0.00
C	220.0	0.00	0.0	0.0	2.44	4.50	0.00	0.00
C	198.9	0.00	0.0	0.0	0.12	0.09	0.00	0.00
C	190.6	0.00	0.0	0.0	0.22	0.60	0.00	0.00
C	180.0	0.00	0.0	0.0	1.87	4.50	0.00	0.00
C	170.0	0.00	0.0	0.0	1.38	4.50	0.00	0.00
C	160.0	0.00	0.0	0.0	1.37	4.50	0.00	0.00
C	122.0	0.00	0.0	0.0	0.05	0.60	0.00	0.00
C	50.0	0.00	0.0	0.0	0.08	0.30	0.00	0.00
D	255.0	0.00	180.0	0.0	0.02	0.03	0.00	0.00
D	240.0	0.00	180.0	0.0	0.02	0.02	0.00	0.00
D	240.0	0.00	180.0	0.0	0.02	0.03	0.00	0.00
D	220.0	0.00	312.7	0.0	0.02	0.03	0.01	-0.01
D	220.0	0.00	312.7	0.0	0.03	0.05	0.02	-0.02
D	200.0	0.00	312.7	0.0	0.03	0.05	0.02	-0.02
D	200.0	0.00	316.1	0.0	0.03	0.06	0.03	-0.02
D	190.0	0.00	318.6	0.0	0.03	0.06	0.02	-0.02
D	190.0	0.00	349.5	0.0	0.03	0.06	0.03	-0.01
D	180.0	0.00	347.7	0.0	0.03	0.06	0.03	-0.01
D	180.0	0.00	25.3	0.0	0.04	0.09	0.04	0.00
D	170.0	0.00	23.8	0.0	0.04	0.09	0.04	0.00
D	170.0	0.00	120.2	0.0	0.05	0.11	0.01	0.00
D	160.0	0.00	120.7	0.0	0.05	0.11	0.01	0.00
D	160.0	0.00	335.8	0.0	0.06	0.14	0.01	-0.01

D	140.0	0.00	329.5	0.0	0.06	0.14	0.01	-0.01
D	140.0	0.00	340.2	0.0	0.06	0.16	0.01	-0.01
D	120.0	0.00	338.5	0.0	0.06	0.16	0.01	-0.01
D	120.0	0.00	341.4	0.0	0.06	0.17	0.02	-0.02
D	100.0	0.00	341.4	0.0	0.06	0.18	0.03	-0.02
D	100.0	0.00	343.5	0.0	0.06	0.18	0.03	-0.02
D	80.0	0.00	342.6	0.0	0.06	0.19	0.03	-0.02
D	80.0	0.00	344.7	0.0	0.06	0.24	0.04	-0.02
D	40.0	0.00	345.7	0.0	0.06	0.25	0.04	-0.02
D	40.0	0.00	346.8	0.0	0.06	0.27	0.05	-0.01
D	0.0	0.00	347.1	0.0	0.06	0.28	0.06	-0.01

ANTENNA LOADING
===============

.....ANTENNA.....			ATTACHMENT		.....ANTENNA FORCES.....			
TYPE	ELEV	AZI	RAD	AZI	AXIAL	SHEAR	GRAVITY	TORSION
	ft		ft		kip	kip	kip	ft-kip
HP	122.0	0.0	8.9	0.0	0.33	0.00	0.28	0.00
HP	106.0	0.0	9.8	0.0	0.32	0.00	0.28	0.00

==========

MAXIMUM MAST DISPLACEMENTS:
==========

ELEV	------DEFLECTIONS (ft)-----			--TILTS (DEG)---		TWIST
ft	NORTH	EAST	DOWN	NORTH	EAST	DEG
255.0	1.218 S	-1.140 J	0.014 S	0.582 S	-0.545 J	-0.041 D
250.0	1.167 S	-1.092 J	0.013 S	0.582 S	-0.545 J	-0.041 D
245.0	1.117 S	-1.045 J	0.013 S	0.582 S	-0.545 J	-0.041 D
240.0	1.066 S	-0.997 J	0.013 S	0.581 S	-0.544 J	-0.041 D
235.0	1.015 S	-0.950 J	0.012 S	0.578 S	-0.542 J	-0.041 D
230.0	0.965 S	-0.903 J	0.012 S	0.574 S	-0.538 J	-0.041 D
225.0	0.914 S	-0.855 J	0.012 S	0.567 S	-0.531 J	-0.041 D
220.0	0.865 S	-0.809 J	0.012 S	0.556 S	-0.521 J	-0.040 D
215.0	0.816 S	-0.763 J	0.011 S	0.547 S	-0.513 J	-0.039 D
210.0	0.768 S	-0.718 J	0.011 S	0.531 S	-0.498 J	-0.038 D
205.0	0.721 S	-0.674 J	0.010 S	0.509 S	-0.477 J	-0.036 D
200.0	0.677 S	-0.633 J	0.010 S	0.480 S	-0.449 J	-0.034 D
195.0	0.635 S	-0.593 J	0.010 S	0.456 S	-0.426 J	-0.031 D
190.0	0.596 S	-0.557 J	0.010 S	0.431 S	-0.404 J	-0.028 D
185.0	0.559 S	-0.522 J	0.009 S	0.407 S	-0.380 J	-0.026 D
180.0	0.524 S	-0.490 J	0.009 S	0.383 S	-0.358 J	-0.024 D
175.0	0.490 S	-0.458 J	0.009 S	0.365 S	-0.342 J	-0.022 D
170.0	0.459 S	-0.429 J	0.008 S	0.348 S	-0.326 J	-0.021 D
165.0	0.428 S	-0.400 J	0.008 S	0.330 S	-0.309 J	-0.019 D
160.0	0.400 S	-0.373 J	0.008 S	0.312 S	-0.292 J	-0.018 D
153.3	0.362 S	-0.338 J	0.007 S	0.294 S	-0.275 J	-0.017 D
146.7	0.329 S	-0.307 J	0.007 S	0.275 S	-0.258 J	-0.015 D
140.0	0.296 S	-0.276 J	0.007 S	0.255 S	-0.239 J	0.015 L
133.3	0.266 S	-0.248 J	0.006 S	0.240 S	-0.225 J	0.014 L
126.7	0.238 S	-0.221 J	0.006 S	0.225 S	-0.210 J	0.014 L
120.0	0.211 S	-0.197 J	0.006 S	0.210 S	-0.196 J	0.014 L
113.3	0.187 S	-0.174 J	0.005 S	0.194 S	-0.181 J	0.013 L
106.7	0.165 S	-0.153 J	0.005 h	0.178 S	-0.167 J	0.012 L
100.0	0.143 S	-0.133 J	0.005 S	0.162 S	-0.152 J	0.010 L
90.0	0.116 S	-0.107 J	0.004 h	0.140 S	-0.130 J	0.009 L
80.0	0.092 S	-0.085 J	0.004 S	0.117 S	-0.109 J	0.007 L
70.0	0.072 S	-0.067 J	0.003 h	0.102 S	-0.095 J	0.006 L

60.0	0.054 S	-0.050 J	0.003 S	0.087 S	-0.081 J	0.005 L
50.0	0.039 S	-0.036 J	0.002 h	0.073 S	-0.068 J	0.004 L
40.0	0.026 S	-0.024 J	0.002 S	0.058 S	-0.054 J	0.003 L
30.0	0.016 S	-0.015 J	0.002 C	0.043 S	-0.040 J	-0.002 D
20.0	0.008 S	-0.008 J	0.001 U	0.029 S	-0.027 J	-0.002 D
10.0	0.003 S	-0.002 J	0.001 C	0.014 S	-0.013 J	-0.001 D
0.0	0.000 A	0.000 A	0.000 A	0.000 A	0.000 A	0.000 A

MAXIMUM ANTENNA AND REFLECTOR ROTATIONS:
==========

ELEV ft	AZI deg	TYPE *	.....BEAM DEFLECTIONS (deg)..... ROLL	YAW	PITCH	TOTAL
122.0	0.0	HP	-0.214 S	0.014 L	0.200 J	0.201 J
106.0	0.0	HP	-0.177 S	0.012 L	0.165 J	0.165 J

MAXIMUM TENSION IN MAST MEMBERS (kip)
==========

ELEV ft	LEGS	DIAG	HORIZ	BRACE
255.0	----------		0.00 C	0.00 A
	0.00 A	0.02 Y		
250.0	----------		0.00 S	0.00 A
	0.05 Y	0.07 V		
245.0	----------		0.00 Y	0.00 A
	0.21 Y	0.10 D		
240.0	----------		0.03 A	0.00 A
	0.49 A	0.17 h		
235.0	----------		0.03 A	0.00 A
	0.93 A	0.28 h		
230.0	----------		0.02 S	0.00 A
	1.53 A	0.40 P		
225.0	----------		0.04 A	0.00 A
	2.49 A	0.45 P		
220.0	----------		0.37 U	0.00 A
	3.47 A	1.61 P		
215.0	----------		0.09 A	0.00 A
	7.57 A	1.78 P		
210.0	----------		0.01 S	0.00 A
	11.77 A	1.80 P		
205.0	----------		0.10 A	0.00 A
	16.12 A	1.89 P		
200.0	----------		0.06 S	0.00 A
	19.47 A	1.08 A		
195.0	----------		0.10 A	0.00 A
	21.91 A	1.08 S		
190.0	----------		0.01 g	0.00 A
	23.77 A	1.03 A		
185.0	----------		0.10 A	0.00 A
	25.89 A	1.04 R		
180.0	----------		0.03 I	0.00 A
	26.80 A	1.71 A		
175.0	----------		0.10 A	0.00 A
	30.16 A	1.72 S		
170.0	----------		0.03 I	0.00 A
	31.64 A	2.07 A		

165.0	----------		0.11 A	0.00 A
	35.44 A	2.14 S		
160.0	----------		0.03 I	0.00 A
	37.80 A	2.64 A		
153.3	----------		0.09 A	0.00 A
	43.20 A	2.73 S		
146.7	----------		0.01 I	0.00 A
	47.40 A	2.45 R		
140.0	----------		0.07 A	0.00 A
	51.92 A	2.58 j		
133.3	----------		0.01 L	0.00 A
	55.63 A	2.39 R		
126.7	----------		0.05 A	0.00 A
	59.51 A	2.53 R		
120.0	----------		0.01 S	0.00 A
	62.66 A	2.58 R		
113.3	----------		0.07 A	0.00 A
	66.37 A	2.65 j		
106.7	----------		0.01 S	0.00 A
	69.63 A	2.73 R		
100.0	----------		0.05 M	0.00 A
	73.81 A	3.18 j		
90.0	----------		0.04 A	0.00 A
	78.85 A	2.98 R		
80.0	----------		0.04 M	0.00 A
	83.39 A	3.15 j		
70.0	----------		0.02 A	0.00 A
	87.81 A	3.02 R		
60.0	----------		0.03 Y	0.00 A
	91.92 A	3.19 j		
50.0	----------		0.02 A	0.00 A
	95.93 A	3.13 R		
40.0	----------		0.03 A	0.00 A
	99.82 A	3.28 j		
30.0	----------		0.02 A	0.00 A
	103.64 A	3.21 R		
20.0	----------		0.00 b	0.00 A
	107.26 A	3.36 j		
10.0	----------		0.02 A	0.00 A
	110.84 A	3.29 R		
0.0	----------		0.00 A	0.00 A

MAXIMUM COMPRESSION IN MAST MEMBERS (kip)
==========

ELEV ft	LEGS	DIAG	HORIZ	BRACE
255.0	----------		0.00 A	0.00 A
	-0.04 H	-0.03 G		
250.0	----------		0.00 A	0.00 A
	-0.21 G	-0.06 e		
245.0	----------		0.00 e	0.00 A
	-0.47 G	-0.12 G		
240.0	----------		0.00 A	0.00 A
	-0.89 S	-0.23 G		

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235.0  ----------------------        -0.02 S       0.00 A
          -1.64 S      -0.26 P
230.0  ----------------------        -0.01 A       0.00 A
          -2.70 S      -0.42 P
225.0  ----------------------        -0.03 S       0.00 A
          -3.88 S      -0.46 h
220.0  ----------------------        -0.28 C       0.00 A
          -7.90 S      -1.74 P
215.0  ----------------------        -0.05 S       0.00 A
         -12.59 S      -1.72 P
210.0  ----------------------        -0.02 A       0.00 A
         -16.91 S      -1.85 P
205.0  ----------------------        -0.06 S       0.00 A
         -21.65 S      -1.90 P
200.0  ----------------------        -0.11 A       0.00 A
         -25.14 S      -1.26 S
195.0  ----------------------        -0.07 S       0.00 A
         -28.23 S      -1.00 R
190.0  ----------------------         0.00 O       0.00 A
         -30.50 S      -1.15 S
185.0  ----------------------        -0.07 S       0.00 A
         -33.06 S      -1.01 R
180.0  ----------------------        -0.03 a       0.00 A
         -37.08 S      -1.90 S
175.0  ----------------------        -0.07 S       0.00 A
         -41.12 S      -1.62 R
170.0  ----------------------        -0.03 a       0.00 A
         -45.68 S      -2.33 S
165.0  ----------------------        -0.07 S       0.00 A
         -50.29 S      -1.99 R
160.0  ----------------------        -0.03 a       0.00 A
         -55.75 S      -2.97 S
153.3  ----------------------        -0.06 S       0.00 A
         -62.42 S      -2.52 j
146.7  ----------------------        -0.01 a       0.00 A
         -66.91 S      -2.71 S
140.0  ----------------------        -0.04 e       0.00 A
         -72.58 S      -2.44 R
133.3  ----------------------         0.00 C       0.00 A
         -76.83 S      -2.57 S
126.7  ----------------------        -0.03 e       0.00 A
         -82.00 S      -2.53 j
120.0  ----------------------         0.00 A       0.00 A
         -86.52 S      -2.69 j
113.3  ----------------------        -0.04 S       0.00 A
         -91.18 S      -2.61 R
106.7  ----------------------        -0.01 A       0.00 A
         -95.68 S      -2.94 j
100.0  ----------------------        -0.03 S       0.00 A
        -101.15 S      -3.03 R
 90.0  ----------------------        -0.02 e       0.00 A
        -107.31 S      -3.19 j
 80.0  ----------------------        -0.02 S       0.00 A
        -113.63 S      -3.04 R
 70.0  ----------------------        -0.01 e       0.00 A
        -119.65 S      -3.20 j
 60.0  ----------------------        -0.02 S       0.00 A
        -125.74 S      -3.10 R
 50.0  ----------------------        -0.01 G       0.00 A
        -131.67 S      -3.29 j
 40.0  ----------------------        -0.02 S       0.00 A
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        -137.62 S      -3.21 R
 30.0  ----------------------        -0.01 G       0.00 A
        -143.33 S      -3.35 j
 20.0  ----------------------         0.00 J       0.00 A
        -149.11 S      -3.29 R
 10.0  ----------------------        -0.01 S       0.00 A
        -154.64 S      -3.43 j
  0.0  ----------------------         0.00 A       0.00 A
```

MAXIMUM INDIVIDUAL FOUNDATION LOADS: (kip)
==========

--------------LOAD---COMPONENTS--------------				TOTAL
NORTH	EAST	DOWN	UPLIFT	SHEAR
14.12 S	-11.93 G	157.25 S	-112.39 A	14.12 S

MAXIMUM TOTAL LOADS ON FOUNDATION : (kip & kip-ft)
==========

------HORIZONTAL-----			DOWN	---------OVERTURNING---------			TORSION
NORTH	EAST	TOTAL @ 0.0		NORTH	EAST	TOTAL @ 0.0	
21.8 S	-20.0 J	21.8 S	60.3 M	2969.6 S	-2757.2 J	2969.6 S	-8.0 D

==========

Seismic Load Effects
Equivalent Lateral Force Procedure
ANSI/TIA-222-H

Parameters	
Risk Category	III
R	3.000
S_S	0.184
S_1	0.058
Site Class	D (default)
T_L (sec)	12.000
F_a	1.600
F_v	2.400
S_{MS}	0.294
S_{M1}	0.139
S_{DS}	0.196
S_{D1}	0.093
T_s	0.474
I_e	1.250
Ω	1.500
C_S	0.034
h (ft)	255.00
K_f	4,540
W_a (ft)	12.84
W_o (ft)	25.00
W (kips)	65.428
W_1 (kips)	27.073
W_2 (kips)	0.480
f_1 (Hertz)	0.889
T (sec)	1.125
k_e	1.3125
V_s (kips)	2.225
Seismic Design Category	B

Vertical Distribution of Seismic Forces

Description	h_i (ft.)	w_i (kips)	W_2 (kips)	$w_i h_i^{ke}$	F_{sz} or E_h (kips)	E_V (kips)	1.2 D + 1.0 E_V (kips)	0.9 D - 1.0 E_V (kips)
Structure - Section 1	247.50	0.5650	0.4803	782.7371	0.0477	0.0221	0.7001	0.4864
Antenna Load	238.86	0.0880	0.0000	116.3579	0.0071	0.0034	0.1090	0.0758
Mount Load	230.65	0.6000	0.0000	757.7530	0.0462	0.0235	0.7435	0.5165
Structure - Section 2	230.00	0.7210	0.0000	907.2000	0.0553	0.0283	0.8935	0.6206
Ladder/Line	225.32	0.0511	0.0000	62.5850	0.0038	0.0020	0.0633	0.0440
Antenna Load	220.00	4.5000	0.0000	5,341.2423	0.3257	0.1764	5.5764	3.8736
Ladder/Line	210.00	0.2208	0.0000	246.5539	0.0150	0.0087	0.2737	0.1900
Structure - Section 3	210.00	0.9820	0.0000	1,096.5396	0.0669	0.0385	1.2169	0.8453
Antenna Load	198.86	0.0880	0.0000	91.4800	0.0056	0.0034	0.1090	0.0758
Ladder/Line	195.32	0.1032	0.0000	104.7815	0.0064	0.0040	0.1278	0.0889
Mount Load	190.65	0.6000	0.0000	590.1495	0.0360	0.0235	0.7435	0.5165
Structure - Section 4	190.00	1.2300	0.0000	1,204.3957	0.0734	0.0482	1.5242	1.0588
Ladder/Line	185.32	0.1176	0.0000	111.4437	0.0068	0.0046	0.1457	0.1012
Ladder/Line	185.32	0.0511	0.0000	48.4249	0.0030	0.0020	0.0633	0.0440
Antenna Load	180.00	4.5000	0.0000	4,104.4756	0.2503	0.1764	5.5764	3.8736
Ladder/Line	175.00	0.1104	0.0000	97.0413	0.0059	0.0043	0.1368	0.0951
Ladder/Line	175.00	0.1104	0.0000	97.0413	0.0059	0.0043	0.1368	0.0951
Antenna Load	170.00	4.5000	0.0000	3,807.8227	0.2322	0.1764	5.5764	3.8736
Structure - Section 5	170.00	1.7680	0.0000	1,496.0512	0.0912	0.0693	2.1909	1.5219
Ladder/Line	165.00	0.1024	0.0000	83.3197	0.0051	0.0040	0.1269	0.0882
Ladder/Line	165.00	0.1104	0.0000	89.8290	0.0055	0.0043	0.1368	0.0951
Ladder/Line	165.00	0.1104	0.0000	89.8290	0.0055	0.0043	0.1368	0.0951
Antenna Load	160.00	4.5000	0.0000	3,516.5759	0.2144	0.1764	5.5764	3.8736
Ladder/Line	150.00	0.2464	0.0000	176.9133	0.0108	0.0097	0.3054	0.2121
Ladder/Line	150.00	0.2208	0.0000	158.5327	0.0097	0.0087	0.2737	0.1900
Ladder/Line	150.00	0.1632	0.0000	117.1763	0.0071	0.0064	0.2022	0.1405
Ladder/Line	150.00	0.2208	0.0000	158.5327	0.0097	0.0087	0.2737	0.1900
Structure - Section 6	150.00	2.2110	0.0000	1,587.4809	0.0968	0.0867	2.7399	1.9032
Ladder/Line	131.00	0.1987	0.0000	119.4308	0.0073	0.0078	0.2462	0.1710
Ladder/Line	131.00	0.1469	0.0000	88.2959	0.0054	0.0058	0.1821	0.1264
Ladder/Line	131.00	0.1987	0.0000	119.4308	0.0073	0.0078	0.2462	0.1710
Ladder/Line	131.00	0.2218	0.0000	133.3153	0.0081	0.0087	0.2749	0.1909
Structure - Section 7	130.00	2.6990	0.0000	1,606.0296	0.0979	0.1058	3.3446	2.3233
Antenna Load	122.00	0.6000	0.0000	328.4721	0.0200	0.0235	0.7435	0.5165
Antenna Load	122.00	0.1900	0.0000	104.0162	0.0063	0.0074	0.2354	0.1636

Seismic Load Effects
Equivalent Lateral Force Procedure
ANSI/TIA-222-H

Vertical Distribution of Seismic Forces

Description	h_i (ft.)	w_i (kips)	W_2 (kips)	$w_i h_i^{ke}$	F_{sz} or E_h (kips)	E_V (kips)	1.2 D + 1.0 E_V (kips)	0.9 D - 1.0 E_V (kips)
Mount Load	122.00	0.0500	0.0000	27.3727	0.0017	0.0020	0.0620	0.0430
Mount/Antenna Load	122.00	0.2400	0.0000	131.3889	0.0080	0.0094	0.2974	0.2066
Ladder/Line	121.00	0.0221	0.0000	11.9687	0.0007	0.0009	0.0274	0.0190
Ladder/Line	121.00	0.0246	0.0000	13.3227	0.0008	0.0010	0.0305	0.0211
Ladder/Line	121.00	0.0184	0.0000	9.9649	0.0006	0.0007	0.0228	0.0159
Ladder/Line	121.00	0.0221	0.0000	11.9687	0.0007	0.0009	0.0274	0.0190
Ladder/Line	113.00	0.1546	0.0000	76.5376	0.0047	0.0061	0.1916	0.1330
Ladder/Line	113.00	0.1725	0.0000	85.3993	0.0052	0.0068	0.2138	0.1484
Ladder/Line	113.00	0.1288	0.0000	63.7648	0.0039	0.0050	0.1596	0.1109
Ladder/Line	113.00	0.1546	0.0000	76.5376	0.0047	0.0061	0.1916	0.1330
Structure - Section 8	110.00	3.1450	0.0000	1,502.9639	0.0916	0.1233	3.8973	2.7072
Antenna Load	106.00	0.1900	0.0000	86.4903	0.0053	0.0074	0.2354	0.1636
Mount Load	106.00	0.0500	0.0000	22.7606	0.0014	0.0020	0.0620	0.0430
Mount/Antenna Load	106.00	0.2400	0.0000	109.2509	0.0067	0.0094	0.2974	0.2066
Ladder/Line	103.00	0.0739	0.0000	32.3961	0.0020	0.0029	0.0916	0.0636
Ladder/Line	103.00	0.0662	0.0000	29.0206	0.0018	0.0026	0.0820	0.0570
Ladder/Line	103.00	0.0614	0.0000	26.9164	0.0016	0.0024	0.0761	0.0529
Ladder/Line	103.00	0.0662	0.0000	29.0206	0.0018	0.0026	0.0820	0.0570
Ladder/Line	90.00	0.2048	0.0000	75.2096	0.0046	0.0080	0.2538	0.1763
Ladder/Line	90.00	0.2208	0.0000	81.0853	0.0049	0.0087	0.2737	0.1900
Ladder/Line	90.00	0.2208	0.0000	81.0853	0.0049	0.0087	0.2737	0.1900
Ladder/Line	90.00	0.2464	0.0000	90.4865	0.0055	0.0097	0.3054	0.2121
Structure - Section 9	90.00	3.2740	0.0000	1,202.3253	0.0733	0.1283	4.0571	2.8183
Ladder/Line	70.00	0.2208	0.0000	58.3029	0.0036	0.0087	0.2737	0.1900
Ladder/Line	70.00	0.2464	0.0000	65.0627	0.0040	0.0097	0.3054	0.2121
Ladder/Line	70.00	0.2048	0.0000	54.0781	0.0033	0.0080	0.2538	0.1763
Ladder/Line	70.00	0.2208	0.0000	58.3029	0.0036	0.0087	0.2737	0.1900
Structure - Section 10	70.00	4.5180	0.0000	1,192.9917	0.0727	0.1771	5.5987	3.8891
Ladder/Line	55.00	0.1232	0.0000	23.7048	0.0014	0.0048	0.1526	0.1061
Ladder/Line	55.00	0.1024	0.0000	19.7027	0.0012	0.0040	0.1269	0.0882
Ladder/Line	55.00	0.1104	0.0000	21.2420	0.0013	0.0043	0.1368	0.0951
Ladder/Line	55.00	0.1104	0.0000	21.2420	0.0013	0.0043	0.1368	0.0951
Mount/Antenna Load	50.00	0.3020	0.0000	51.2749	0.0031	0.0118	0.3742	0.2600
Structure - Section 11	50.00	4.6540	0.0000	790.1772	0.0482	0.1824	5.7672	4.0062
Ladder/Line	45.00	0.1104	0.0000	16.3234	0.0010	0.0043	0.1368	0.0951
Ladder/Line	45.00	0.1075	0.0000	15.8946	0.0010	0.0042	0.1332	0.0926
Ladder/Line	45.00	0.1232	0.0000	18.2160	0.0011	0.0048	0.1526	0.1061
Ladder/Line	45.00	0.1104	0.0000	16.3234	0.0010	0.0043	0.1368	0.0951
Ladder/Line	30.00	0.2150	0.0000	18.6707	0.0011	0.0084	0.2664	0.1851
Ladder/Line	30.00	0.2208	0.0000	19.1744	0.0012	0.0087	0.2737	0.1900

Seismic Load Effects
Equivalent Lateral Force Procedure
ANSI/TIA-222-H

Vertical Distribution of Seismic Forces

Description	h_i (ft.)	w_i (kips)	W_2 (kips)	$w_i h_i^{ke}$	F_{sz} or E_h (kips)	E_V (kips)	1.2 D + 1.0 E_V (kips)	0.9 D - 1.0 E_V (kips)
Ladder/Line	30.00	0.2464	0.0000	21.3975	0.0013	0.0097	0.3054	0.2121
Ladder/Line	30.00	0.2208	0.0000	19.1744	0.0012	0.0087	0.2737	0.1900
Structure - Section 12	30.00	5.0130	0.0000	435.3308	0.0265	0.1965	6.2121	4.3152
Ladder/Line	10.00	0.2208	0.0000	4.5342	0.0003	0.0087	0.2737	0.1900
Ladder/Line	10.00	0.2150	0.0000	4.4151	0.0003	0.0084	0.2664	0.1851
Ladder/Line	10.00	0.2208	0.0000	4.5342	0.0003	0.0087	0.2737	0.1900
Ladder/Line	10.00	0.2464	0.0000	5.0599	0.0003	0.0097	0.3054	0.2121
Structure - Section 13	10.00	5.2500	0.0000	107.8101	0.0066	0.2058	6.5058	4.5192
	Σ	65.43	0.4803	36,481.43	2.22	2.56	81.08	56.32

Leg Connection Details												
Bottom Elevation (ft)	Top Elevation (ft)	Pipe Dimensions	Top Splice					Bottom Splice/Base				
			Bolt Qty.	Bolt Dia. (in)	Bolt Circle (in)	Plate Thickness (in)	Plate Dia. (in)	Bolt Qty.	Bolt Dia. (in)	Bolt Circle (in)	Plate Thickness (in)	Plate Dia. (in)
240	255	2.375 OD X .154						6	0.75	6.50	0.75	8.50
220	240	2.375 OD X .154	6	0.75	6.50	0.75	8.50	6	0.75	6.50	0.75	8.50
200	220	2.875 OD X .276	6	0.75	6.50	1.00	8.50	6	0.75	6.50	1.00	8.50
180	200	3.500 OD X .300	6	0.75	6.50	1.00	8.50	6	1.00	9.00	1.25	11.50
160	180	4.500 OD X .337	6	1.00	9.00	1.25	11.50	6	1.00	9.00	1.25	11.50
140	160	5.563 OD X .375	6	1.00	9.00	1.25	11.50	6	1.00	9.00	1.25	11.50
120	140	5.563 OD X .500	6	1.00	9.00	1.25	11.50	6	1.00	9.00	1.25	11.50
100	120	5.563 OD X .500	6	1.00	9.00	1.25	11.50	6	1.25	12.50	1.75	15.75
80	100	8.625 OD X .322	6	1.25	12.50	1.50	15.75	6	1.25	12.50	1.50	15.75
60	80	8.625 OD X .500	6	1.25	12.50	1.50	15.75	6	1.25	12.50	1.50	15.75
40	60	8.625 OD X .500	6	1.25	12.50	1.50	15.75	6	1.25	12.50	1.50	15.75
20	40	8.625 OD X .500	6	1.25	12.50	1.50	15.75	6	1.25	12.50	1.50	15.75
0	20	8.625 OD X .500	6	1.25	12.50	1.50	15.75	6	1.25	12.75	1.50	16.00

Diagonal Bracing Connection Details								
Bottom Elevation (ft)	Top Elevation (ft)	Angle Shape	Bolt Qty.	Bolt Dia. (in)	Bolt End Distance (in)	Bolt Spacing (in)	Gage Distance From Heel (in)	Gusset Plate Thickness (in)
240	255	L 2 X 2 X 1/8	1	0.625	1.500		1.125	0.375
220	240	L 2 X 2 X 1/8	1	0.625	1.500		1.125	0.375
200	220	L 2 X 2 X 1/8	1	0.625	1.500		1.125	0.375
180	200	L 2 X 2 X 1/8	1	0.625	1.500		1.125	0.375
160	180	L 2 X 2 X 3/16	1	0.625	1.500		1.125	0.375
140	160	L 2 1/2 X 2 1/2 X 3/16	1	0.625	1.500		1.375	0.375
120	140	L 2 1/2 X 2 1/2 X 3/16	1	0.625	1.500		1.375	0.375
100	120	L 3 X 3 X 3/16	1	0.750	1.500		1.750	0.375
80	100	L 3 X 3 X 1/4	1	0.750	1.625		1.750	0.375
60	80	L 3 1/2 X 3 1/2 X 1/4	1	0.750	1.625		1.750	0.375
40	60	L 3 1/2 X 3 1/2 X 1/4	1	0.750	1.625		1.750	0.375
20	40	L 4 X 4 X 1/4	1	0.750	1.625		2.000	0.375
0	20	L 4 X 4 X 1/4	2	0.625	1.625	2.1250	2.000	0.500

MAT FOUNDATION DESIGN BY SABRE INDUSTRIES

255' S3TL Series HD2 MILESTONE COMMUNICATIONS, INC. Stockton Lake, VA (26-3777-TLJ) 2026-02-04 ARH

Overall Loads:			
Factored Moment (ft-kips)	12170.50		
Factored Axial (kips)	219.60		
Factored Shear (kips)	89.31		
Individual Leg Loads:		Tower eccentric from mat (ft)=	2.25
Factored Uplift (kips)	445.00		
Factored Download (kips)	510.00		
Factored Shear (kips)	53.00		
Width of Tower (ft)	29	Allowable Bearing Pressure (ksf)	**2.50**
Ultimate Bearing Pressure	5.00	Safety Factor	**2.00**
Bearing Φs	0.75		
Bearing Design Strength (ksf)	3.75	Max. Factored Net Bearing Pressure (ksf)	3.52
Water Table Below Grade (ft)	**999**		
Width of Mat (ft)	36	Minimum Mat Width (ft)	35.33
Thickness of Mat (ft)	1.5		
Depth to Bottom of Slab (ft)	6		
Bolt Circle Diameter (in)	12.75		
Effective Anchor Bolt Embedment	52.125		
Diameter of Pier (ft)	4	Minimum Pier Diameter (ft)	2.56
Ht. of Pier Above Ground (ft)	0.5	Equivalent Square b (ft)	3.54
Ht. of Pier Below Ground (ft)	4.5		
Quantity of Bars in Mat	63		
Bar Diameter in Mat (in)	1.27		
Area of Bars in Mat (in^2)	79.81		
Spacing of Bars in Mat (in)	6.85	Recommended Spacing (in)	6 to 12
Quantity of Bars Pier	24		
Bar Diameter in Pier (in)	0.875		
Tie Bar Diameter in Pier (in)	0.5		
Spacing of Ties (in)	4		
Area of Bars in Pier (in2)	14.43	Minimum Pier A_s (in^2)	9.05
Spacing of Bars in Pier (in)	5.24	Recommended Spacing (in)	5 to 12
f'c (ksi)	4.5		
fy (ksi)	60		
Unit Wt. of Soil (kcf)	0.11		
Unit Wt. of Concrete (kcf)	0.15		
Volume of Concrete (yd^3)	78.98		

MAT FOUNDATION DESIGN BY SABRE INDUSTRIES (CONTINUED)

Two-Way Shear:			
Average d (in)	13.73		
ϕv_c (ksi)	0.201	v_u (ksi)	0.169
$\phi v_c = \phi(2 + 4/\beta_c)f'_c{}^{1/2}$	0.302		
$\phi v_c = \phi(\alpha_s d/b_o+2)f'_c{}^{1/2}$	0.223		
$\phi v_c = \phi 4f'_c{}^{1/2}$	0.201		
Shear perimeter, b_o (in)	225.08		
β_c	1		
Stability:			
Overturning Design Strength (ft-k)	16032.9	Factored Overturning Moment (ft-k)	12751.0
One-Way Shear:			
ϕV_c (kips)	596.8	V_u (kips)	559.4
Pier Design:			
Design Tensile Strength (kips)	779.3	Tu (kips)	445.0
Shear:			
ϕ	0.75		
V_c (kips)	125.7		
V_s (kips)	226.2	$V_{s,max}$ (kips)	989.2
ϕV_n (kips)	263.9	V_u (kips)	53.0
Maximum Spacing (in)	9.76	(Only if Shear Ties are Required)	
Actual Hook Development (in)	12.46	Req'd Hook Development l_{dh} (in) - Tension	10.96
		Req'd Hook Development l_{dc} (in) - Compression	11.81
Anchor Bolt Pull-Out:			
$N_{ua} / \phi N_n$	0.94	$V_{ua} / \phi V_n$	0.23
Pier Rebar Development Length (in)	39.54	Required Length of Development (in)	23.48
Flexure in Slab:			
ϕM_n (ft-kips)	4410.5	M_u (ft-kips)	4359.7
a (in)	2.90		
Steel Ratio	0.01345		
β_1	0.825		
Maximum Steel Ratio (ρ_t)	0.0197		
Minimum Steel Ratio	0.0018		

Condition	1 is OK, 0 Fails
Minimum Mat Width	1
Maximum Soil Bearing Pressure	1
Pier Area of Steel	1
Pier Shear	1
Two-Way Shear	1
Overturning	1
Anchor Bolt Pull-Out	1
Flexure	1
Steel Ratio	1
Interaction Diagram	1
One-Way Shear	1
Hook Development	1
Minimum Mat Depth	1
Anchor Bolt Punching Shear	1

DRILLED STRAIGHT PIER DESIGN BY SABRE INDUSTRIES

255' S3TL Series HD2 MILESTONE COMMUNICATIONS, INC. Stockton Lake, VA (26-3777-TLJ) 2026-02-04

Factored Uplift (kips)	445.00
Factored Download (kips)	510.00
Factored Shear (kips)	53.00
Ultimate Bearing Pressure	9
Bearing ϕ_s	0.75
Bearing Design Strength (ksf)	6.75
Water Table Below Grade (ft)	999
Bolt Circle Diameter (in)	12.75
Effective Anchor Bolt Embedment	52.125
Pier Diameter (ft)	5.5
Ht. Above Ground (ft)	0.5
Pier Length Below Ground (ft)	66
Quantity of Bars	18
Bar Diameter (in)	1.128
Area of Bars (in^2)	17.99
Spacing of Bars (in)	10.05
Tie Bar Diameter (in)	0.5
Spacing of Ties (in)	12
f'_c (ksi)	4.5
f_y (ksi)	60
Unit Wt. of Concrete (kcf)	0.15
Volume of Concrete (yd^3)	58.52

Minimum Pier Diameter (ft): 2.56

Minimum Area of Steel (in^2): 17.11

Length to ignore download (ft): 0

Ignore bottom length in download? ☐

Depth at Bottom of Layer (ft)	Ult. Skin Friction (ksf)	(Ult. Skin Friction)*(Uplift Factor)	γ (kcf)
82.5	0.50	0.50	0.11

DRILLED STRAIGHT PIER DESIGN BY SABRE INDUSTRIES (CONTINUED)

Download:			
Φ_s, Download Friction	0.75		
Q_f, Skin Friction (kips)	570.2	W_s (kips)	172.5
Q_b, End Bearing Strength (kips)	213.8	W_c (kips)	237.0
Download Design Strength (kips)	588.0	Factored Net Download (kips)	587.4
Uplift (skin friction):			
Φ_s, Uplift (friction)	0.75		
Q_f, Skin Friction (kips)	570.2		
W_c (kips)	237.0		
W_w (kips)	0.0		
Uplift Design Strength (kips)	640.9	Factored Uplift (kips)	445.0
Uplift (cone):			
Φ_s, Uplift (cone)	0.75		
$W_{s,cone}$ (kips)	13429.1		
$W_{w,cone}$ (kips)	0.0		
W_c (kips)	237.0		
$W_{w,cyl}$ (kips)	0.0		
Uplift Design Strength (kips)	10285.1	Factored Uplift (kips)	445.0
Tension:			
Design Tensile Strength (kips)	971.3	T_u (kips)	445.0
Shear:			
ϕ	0.75		
V_c (kips)	345.9		
V_s (kips)	103.7	$V_{s,max}$ (kips)	1870.1
ϕV_n (kips)	337.2	V_u (kips)	53.0
Anchor Bolt Pull-Out:			
$N_{ua} / \phi N_n$	0.94	$V_{ua} / \phi V_n$	0.23
Rebar Development Length (in)	33.33	Required Length of Development (in)	30.27

Condition	1 is OK, 0 Fails
Download	1
Uplift	1
Area of Steel	1
Shear	1
Anchor Bolt Pull-Out	1
Interaction Diagram	1

Exhibit “8”

February 4, 2026

Matt Penning
Milestone Communications, Inc.
121100Sunset Hills Road #600
Reston, VA 20190

RE: Proposed 255' Sabre Self-Supporting Tower for Stockton Lake, VA

Dear Mr. Russo,

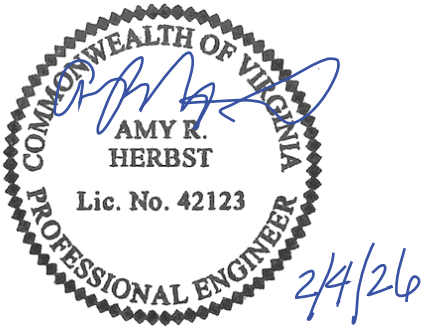
Upon receipt of order, we propose to design and supply a tower for the above referenced project for an Ultimate Wind Speed of 119 mph with no ice and 30 mph with 1.5" ice, Risk Category III, Exposure C and Topographic Category 1 in accordance with the Telecommunications Industry Association Standard ANSI/TIA-222-H, "Structural Standard for Antenna Supporting Structures, Antennas and Small Wind Turbine Support Structures".

When designed according to this standard, the wind pressures and steel strength capacities include several safety factors. Therefore, it is highly unlikely that the tower will fail structurally in a wind event where the design wind speed is exceeded within the range of the built-in safety factors.

Should the wind speed increase beyond the capacity of the built-in safety factors, to the point of failure of one or more structural elements, the most likely location of the failure would be within one or more of the tower members in the upper portion. This would result in a buckling failure mode, where the loaded member would bend beyond its elastic limit (beyond the point where the member would return to its original shape upon removal of the wind load).

Therefore, it is likely that the overall effect of such an extreme wind event would be localized buckling of a tower section. Assuming that the wind pressure profile is similar to that used to design the tower, the tower is most likely to buckle at the location of the highest combined stress ratio in the upper portion of the tower. This would result in the portion of the tower above the failure location "folding over" onto the portion of the tower below the failure location. *Please note that this letter only applies to the above referenced tower designed and manufactured by Sabre Industries*. This would effectively result in a fall radius less than or equal to 180'.

Sincerely,



Amy R. Herbst, P.E.
Senior Engineer

Sabre Industries, Inc.

7101 Southbridge Drive
Sioux City, IA 51111

Exhibit “9”

February 9, 2026

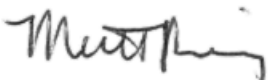
Robert Love
Director
Prince Edward County Planning & Zoning
111 N. South Street, 3rd Floor,
Farmville, VA 23901
(434) 414-3037

Re: Milestone Towers- Site Name: Stockton Lake- Site Location: 4451 Five Forks Road, Pamplin, VA 23958 (TMS # 046-A-50)- Telecommunication Facility- Collocation Policy Letter

Mr. Love,

Per Section 3-100.13- Towers (C)(10), Milestone Tower Limited Partnership-IV (Milestone Towers) ensures that the proposed telecommunications facility is designed to accommodate at least three (3) wireless broadband carriers. Milestone Towers, any future successors and assigns, shall allow any communication company to collocate on the tower at the prevailing market rate for this facility.

Sincerely,



Matthew Penning

Director of Development
Milestone Towers
matt@milestonetowers.com
703.865.4697 (office)

Exhibit “10”

February 9, 2026

Robert Love
Director
Prince Edward County Planning & Zoning
111 N. South Street, 3rd Floor,
Farmville, VA 23901
(434) 414-3037

Re: Milestone Towers- Site Name: Stockton Lake- Site Location: 4451 Five Forks Road, Pamplin, VA 23958 (TMS # 046-A-50)- Telecommunication Facility- Tower Removal Letter

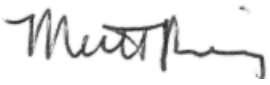
Dear Mr. Love,

Please accept the signed statement below as confirming Section 3-100.13- Towers (C)(16) of the Prince Edward County Code of Ordinances:

Milestone Tower Limited Partnership-IV (Milestone Towers), its successors and assigns, provide this statement declaring itself, its successors and assigns of being financially responsible to assure the proposed communications tower, which is no longer used for communications purposes for a continuous period of ninety (90) days, will be dismantled and removed within ninety (90) days of receipt of notice from the Building Official or County Administrator notifying the owner of such removal requirement.

Please contact me should you have any questions.

Sincerely,



Matthew Penning

Director of Development
Milestone Towers
matt@milestonetowers.com
703.865.4697 (office)

Exhibit “11”

February 9, 2026

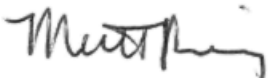
Robert Love
Director
Prince Edward County Planning & Zoning
111 N. South Street, 3rd Floor,
Farmville, VA 23901
(434) 414-3037

Re: Milestone Towers- Site Name: Stockton Lake- Site Location: 4451 Five Forks Road, Pamplin, VA 23958 (TMS # 046-A-50)- Indemnification Letter

Mr. Love,

Except for expenses or liabilities arising from the negligence of the County of Prince Edward, Milestone Tower Limited Partnership-IV (Milestone Towers) hereby expressly agrees to indemnify and hold the County of Prince Edward harmless against expenses and liabilities arising out of the construction and operation of the proposed tower as follows: Milestone Towers expressly agrees to the extent that there is a causal relationship between its negligent, reckless, or intentionally wrongful action or inaction, or the negligent, reckless or intentionally wrongful action or inaction of any of its employees or any person, firm, or corporation directly or indirectly employed by Milestone Towers, and any damage, liability, injury loss or expense (whether in connection with bodily injury or death or property damage or loss) that is suffered by the County of Prince Edward and its employees or any member of the public, to indemnify and save the County of Prince Edward and its employees harmless against any and all liabilities, penalties, demands, claims, lawsuits, losses, damages, costs, and expenses arising out of the performance of the construction or operation of the proposed tower.

Sincerely,



Matthew Penning

Director of Development
Milestone Towers
matt@milestonetowers.com
703.865.4697 (office)

Exhibit “12”

U.S. TITLE SOLUTIONS
File No. UST75300 Reference No. 5000918898

REPORT OF TITLE
SCHEDULE - II

(LEGAL DESCRIPTION)

Parcel No. Two: ALL THOSE TWO CERTAIN TRACTS, pieces or parcels of land with the buildings and improvements thereon, situate, lying and being in Buffalo Magisterial District, Prince Edward County, Virginia, one said parcel containing 50.00 acres, more or less, adjoining the lands now or formerly owned by S. T. Harris, A. W. Covington and J. H. Crute, et als; and the other said parcel containing 6.00 acres, more or less, adjoining lands now or formerly owned by A. W. Covington and D. T. Hancock, et als; LESS HOWEVER that certain off conveyance made to the Commonwealth of Virginia for highway purposes with said Deeds recorded in the above mentioned Clerk's Office in Deed Book 127 at page 59 and Deed Book 164 at page 164; and being the same property conveyed to the above grantor by deed of Alma LeFerne Gaunce Tutwiler, dated June 22, 1976 and recorded in the above mentioned Clerk's Office in Deed Book 330 at page 884. Reference to said Deed is hereby expressly given for a more accurate and complete description of said property.

**Please note: the above description was taken from the vesting deed in Book 354, Page 211. Said description contains a reference to an outsale in Deed Book 127, Page 59. However, said Deed is not an outsale of the subject parcel.

Exhibit “13”

This Deed was prepared without the
benefit of a title exmaination BOOK 0354 PAGE 213

TAX MAP NO: 46--A--45
TAX MAP NO: 46--A--50

JAMES ALTON TURNS

TO:

DEED OF GIFT

ROBERT ALTON GAUNCE

THIS DEED, Made this 28th day of March, 2001, by and between JAMES ALTON TURNS, GRANTOR, party of the first part, and ROBERT ALTON GAUNCE, GRANTEE, party of the second part. Pursuant to Virginia Code Section 58.1-811 D, 1950 (as amended), no recordation taxes shall be required for the recordation of this Deed of Gift because no monetary consideration has passed between the parties

W I T N E S S E T H :

The grantor does for and in consideration of the natural love and affection of the grantor for the grantee, hereby grant, give and convey in fee simple with Special Warranty unto the party of the second part, the following described property, to-wit: wit:

Parcel No. One: ALL THAT CERTAIN TRACT, piece or parcel of land with the buildings and improvements thereon, containing 10.00 acres, situate, lying and being in Buffalo Magisterial District, Prince Edward County, Virginia, located on the southwest side of Virginia Secondary Highway No. 658 and more clearly shown on that certain Plat made by Meredith Helm, C.L.S., dated April 2, 1975 and recorded in the Clerk's Office of the Circuit Court of Prince Edward County, Virginia in Deed Book 196, at page 50. Reference to said Plat is hereby expressly given for a more accurate and complete description of said property; and being the same property conveyed to the above grantor, also known as J. Alton Turns, by deed dated July 16, 1975 and recorded in the above mentioned Clerk's Office in Deed Book 196, at page 45.

Parcel No. Two: ALL THOSE TWO CERTAIN TRACTS, pieces or

JAMES L. WHITLOCK
ATTORNEY AT LAW
100 EAST THIRD STREET
FARMVILLE, VIRGINIA 23901

parcels of land with the buildings and improvements thereon, situate, lying and being in Buffalo Magisterial District, Prince Edward County, Virginia, one said parcel containing 50.00 acres, more or less, adjoining the lands now or formerly owned by S. T. Harris, A. W. Covington and J. H. Crute, et als; and the other said parcel containing 6.00 acres, more or less, adjoining lands now or formerly owned by A. W. Covington and D. T. Hancock, et als; LESS HOWEVER that certain off conveyance made to the Commonwealth of Virginia for highway purposes with said Deeds recorded in the above mentioned Clerk's Office in Deed Book 127 at page 59 and Deed Book 164 at page 164; and being the same property conveyed to the above grantor by deed of Alma LeFerne Gaunce Tutwiler, dated June 22, 1976 and recorded in the above mentioned Clerk's Office in Deed Book 330 at page 884. Reference to said Deed is hereby expressly given for a more accurate and complete description of said property.

This conveyance is expressly made subject to all valid reservations, restrictions, conditions and easements of record in the chain of title, if any, constituting constructive notice.

WITNESS the following signature and seal:

James Alton Turns (SEAL)

STATE OF VIRGINIA,

COUNTY OF PRINCE EDWARD, to-wit:

The foregoing instrument was acknowledged before me this 5th day of April, 2001, by JAMES ALTON TURNS, in the County and State aforesaid. Farmville VA

MY Commission expires: 12-31-02

Notary Public

JAMES L. WHITLOCK
ATTORNEY AT LAW
100 EAST THIRD STREET
FARMVILLE, VIRGINIA 23901

St. R. Tax	(039)	
Co. R. Tax	(213)	
Transfer	(212)	1 00
Clerk	(301)	12 00
Lib.	(145)	1 00
Tech T.F.	(106)	3 00
Grantor Tax	(212/220)	
Total $		17 00

VIRGINIA: CLERK'S OFFICE OF THE CIRCUIT COURT OF PRINCE EDWARD COUNTY

The foregoing instrument with acknowledgement was admitted to record on April 9th 2001 at 10:50 A.M. in D.B. 354 Page(s) 211

Teste: Frank L. Overton, Clerk

By: Regina Williams, Dep. Clerk

Grantor's Tax: $

Examined and Mailed/Delivered To: James Whitlock

Date: 4-9-01

Form R.W. 28-1
Rev. 6-17-64

O. H. Turner

BOOK 164 PAGE 164

Turns
To: Agreement
Commonwealth of Virginia
Tax Exempt

THIS AGREEMENT, made this 25 day of MAY, 1967, by and between O. H. Turner, widower of Prince Edward County, Virginia, party of the first part (even though more than one), and the Commonwealth of Virginia, party of the second part,

WITNESSETH: THAT, WHEREAS, it is proposed by the Commonwealth to widen or improve State Highway Route 626, Project 0626-073-120,C-501, From 0.047 Mi. W. Bell Creek To 0.066 Mi. E. Bell Creek, in Prince Edward County, Virginia; and

WHEREAS, In the improvement it is necessary that the said party of the second part extend the road slopes and/or other construction on to the lands of the party of the first part from Sta. 10+48 to Sta. 10+55 of drainage traverse as shown on Sheet 3 of the plans for the above said Project on file in the office of the Department of Highways, Richmond, Virginia;

NOW, THEREFORE, For and in consideration of $1.00 in hand paid to the said party of the first part, receipt of which is hereby acknowledged, the party of the first part doth grant and convey to the party of the second part, with general warranty, the right and easement to use the additional areas from Sta. 10+48 to Sta. 10+55 of drainage traverse and as shown outlined in green on a photo copy of said Sheet 3, for the proper execution and maintenance of the work, free from encroachments, which photo copy is hereto attached as a part hereof and is to be recorded simultaneously herewith in the State Highway Plat Book.

Drainage Easement and any and all damages - donated.

The party of the first part covenants and agrees for himself, his heirs, successors and assigns, that the consideration hereinabove mentioned and paid to him shall be in lieu of any and all claims to compensation for said easement, and for damages, if any, which may result by reason of the use to which the grantee will put the easement to be conveyed.

The party of the first part covenants that he is seized of the land in fee simple out of which this easement is conveyed; that he has the right to convey the said easement to the party of the second part; that he has done no act to encumber the said land conveyed in easement; that the party of the second part shall have quiet possession of the easement, free from all encumbrances, and that he will execute such further assurances of the said easement as may be requisite.

Form R.W. 28-2

BOOK 164 PAGE 165

WITNESS the following signature(s) and seal(s):

O. H. Twins (SEAL)

_____ (SEAL)

STATE OF VIRGINIA,

COUNTY OF Prince Edward, To-wit:

I, Ray E. White, a Notary Public, in and for the State of Virginia, at large, do certify that O. H. TWINS and _____, whose names are signed to the foregoing and annexed writing, bearing date on the 25 day of MAY, 1967, have acknowledged the same before me in the County aforesaid.

My Commission expires 10/26/70.

Given under my hand this 25 day of MAY, 1967

Ray E. White
Notary Public

Plat recorded in State Highway
Plat Book #2 at page 118.

Virginia:
In the Clerk's Office of the Circuit Court of Prince Edward County, July 20,1967.
This deed was this day presented in said office and, with certificate of acknowledgment annexed, admitted to record at 9 o'clock A.M., and indexed.

Teste:

Vernon C. Womack, Clerk

Examined and mailed to
Mr. Alvah S. Mattox
State Right of Way Engineer
Virginia Department of Highways
1221 E. Broad Street
Richmond, Va., 23219

Exhibit “14”

THIS INSTRUMENT PREPARED BY & TO BE RETURNED TO:
J. Ladd Johnson
Edinger Associates PLLC
1725 I St. NW, Suite 300
Washington DC, 20006

Tax Parcel No.: 046 A 50
Consideration: $5.00

MEMORANDUM OF LEASE

THIS MEMORANDUM OF LEASE is entered into as of the 13th day of July, 2023, by and between Milestone Tower Limited Partnership-IV, a Delaware limited partnership ("Lessee"), and ROBERT ALTON GAUNCE ("Lessor").

RECITALS:

A. Lessor and Lessee are parties to a Site Lease Agreement, dated July 13th, 2023 (the "Lease"), pursuant to which Lessor has leased to Lessee certain real property in Prince Edward County, Virginia described in Exhibit "A" attached hereto.

B. Lessor and Lessee wish to enter into this Memorandum of Lease.

NOW, THEREFORE, in consideration of the premises, the sum of Five Dollars ($5.00) and other good and valuable consideration, the receipt and sufficiency of which is hereby acknowledged, Lessor and Lessee hereby agree as follows:

1. The name of the lessor under the Lease is ROBERT ALTON GAUNCE.

2. The name of the lessee under the Lease is Milestone Tower Limited Partnership-IV.

3. The address of Lessor, as stated in the Lease, is 1421 Commerce Road, Verona, VA 24482. The address of Lessee, as stated in the Lease, is Milestone Communications, 12110 Sunset Hills Road, Suite 600, Reston, VA 20190.

4. The leased premises, as described in the Lease, consists of a portion of the property owned by the Lessor located at 4451 Five Forks Road, Pamplin, VA 23958, and known as Parcel ID 046 A 50 and as more particularly described on the attached Exhibit "A".

5. The term of the Lease is ten (10) years. The date of commencement of the term of the Lease was July 13th, 2023, and the date of termination of the term of the Lease is ten (10) years thereafter, subject to any applicable renewal period.

6. Provided Lessee is not in default under the Lease beyond any applicable cure period, Lessee may renew the Lease for five (5) five-year renewal periods, to commence at the end of the initial term of the Lease. Accordingly, the latest date to which the term of the Lease may be extended is July 13, 2058.

(Signatures continue on the following 2 pages)

IN WITNESS WHEREOF, the undersigned LESSEE has duly executed this Memorandum of Lease under seal as of the first date stated above.

ATTEST:

MILESTONE TOWER LIMITED PARTNERSHIP - IV, a Delaware limited partnership

By: **MILESTONE COMMUNICATIONS MANAGEMENT IV, INC.,** a Delaware limited liability company, its general partner

By: _____
Leonard Forkas, Jr., President
Date: July 13, 2023

STATE OF Virginia)
) **TO WIT:**
COUNTY OF Fairfax)

I hereby certify that on this 13 day of JULY, 2023 before me, a Notary Public for the state and county aforesaid, personally appeared Leonard Forkas, Jr., known to me or satisfactorily proven to be the person whose name is subscribed to the foregoing instrument, and acknowledged that he executed the foregoing instrument, acting in his capacity as President of MILESTONE COMMUNICATIONS MANAGEMENT IV, INC., the general partner of MILESTONE TOWER LIMITED PARTNERSHIP-IV, for the purposes therein set forth.

RUTH GETACHEW MENKIR
COMMONWEALTH OF VIRGINIA
REGISTRATION NO. 8037208
MY COMM. EXPIRES 10-31-27
NOTARY PUBLIC

[SEAL]

_____
Notary Public

My Commission Expires: 10·31·2027

Notary Registration #: 8037208

IN WITNESS WHEREOF, the undersigned LESSOR has duly executed this Memorandum of Lease under seal as of the first date stated above.

ATTEST:

_____

Robert Alton Gaunce

ROBERT ALTON GAUNCE,
an individual

STATE OF VIRGINIA AT LARGE)

COUNTY OF NELSON) **TO WIT:**

I hereby certify that on this 17th day of July, 2023, before me, a Notary Public for the state and county aforesaid, personally appeared ROBERT ALTON GAUNCE, known to me or satisfactorily proven to be the person whose name is subscribed to the foregoing instrument, and acknowledged that he executed the foregoing instrument, for the purposes therein set forth.

Notary Public

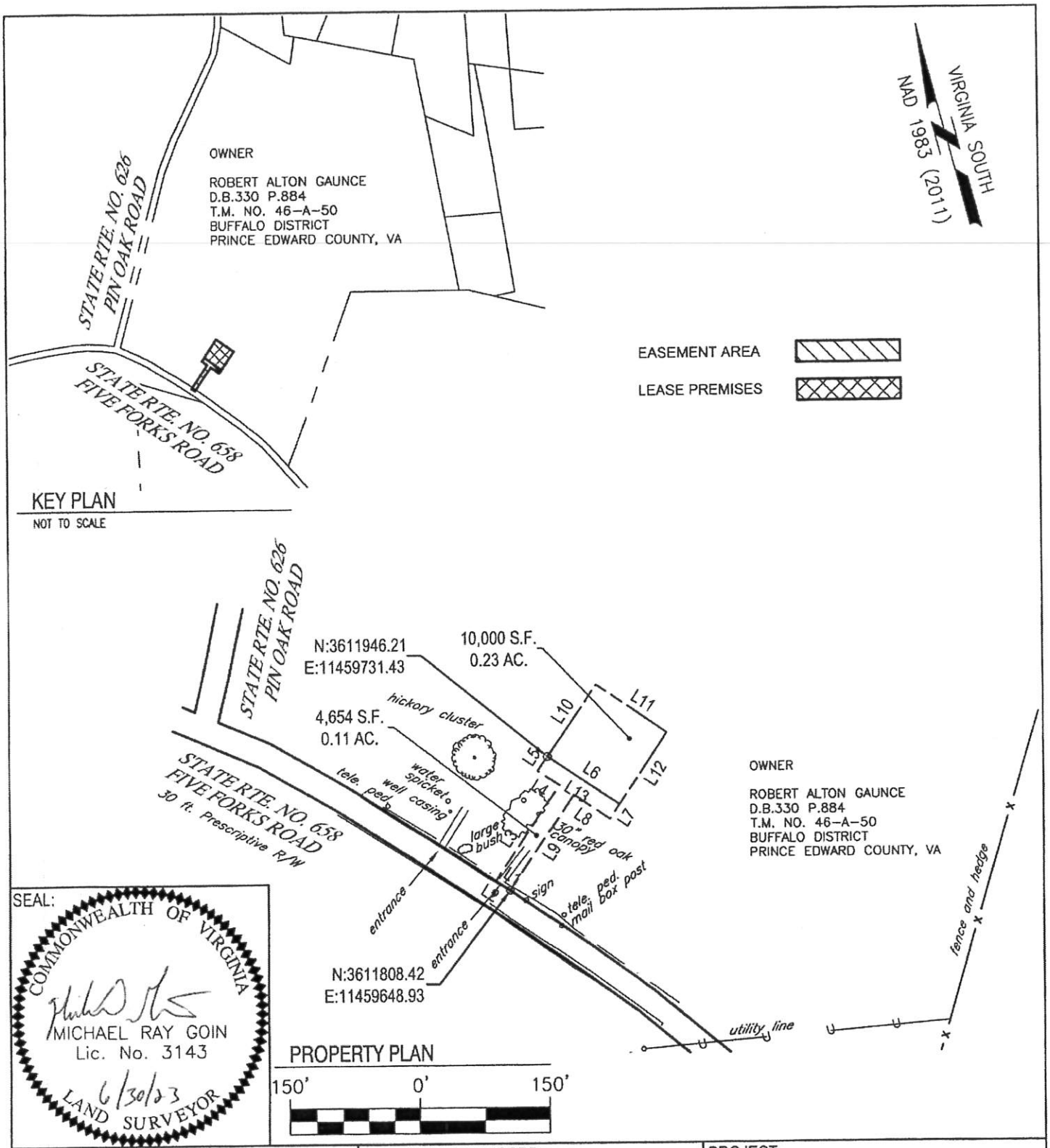
[SEAL]

My Commission Expires: 3/31/25

Registration #: 182488

Exhibit A to
Memorandum of Lease

(Legal Description)



SEAL:

COMMONWEALTH OF VIRGINIA
MICHAEL RAY GOIN
Lic. No. 3143
6/30/23
LAND SURVEYOR

NOTES:

1. PROPERTY LINES SHOWN BASED ON RECORDED DEEDS AND PLATS. THIS IS NOT A BOUNDARY SURVEY.
2. PROJECT IS UNDER THE JURISDICTION OF PRINCE EDWARD COUNTY, VA.
3. SURVEY WAS PERFORMED ON 4/27/23 USING A SOKIA GRX3 RECEIVER WITH TOPCON NETWORK TO ESTABLISH CONTROL. DATA WAS COLLECTED WITH SOKIA ROBOT.

SURVEYOR OF RECORD:

DICKERSON SURVEYING LLC
500 Court Street P.O. Box 112
Appomattox, Virginia 24522
434-352-8560
Michael Ray Goin
Land Surveyor

ENGINEER OF RECORD:

BAMMAN CONSULTING, LLC
14489 ST ANDREWS LN
ASHLAND, VA 23005
PH: 703.328.8574

PROJECT:

STOCKTON LAKE

SITE INFORMATION:

4451 FIVE FORKS ROAD
PAMPLIN, VA 23958

SHEET NO:

1 OF 3

20' WIDE INGRESS/EGRESS & UTILITY/FIBER EASEMENT DESCRIPTION

BEGINNING AT A POINT IN THE CENTERLINE INTERSECTION OF STATE ROUTE NO. 658, FIVE FORKS ROAD (30' PRESCRIPTIVE EASEMENT) AND STATE ROUTE NO. 626, PIN OAK ROAD, MARKED WITH A MAG NAIL. SAID POINT OF BEGINNING HAVING A VA SOUTH STATE PLANE GRID COORDINATE OF N:3612082.47, E:11459357.34 NAD 83(2011)(US SURVEY FEET).

THENCE FROM THE BEGINNING POINT
S46°46'32"E 400.16'
TO THE BEGINNING OF THE LEASE PREMESIS AT A POINT ON THE NORTHEASTERN EDGE OF THE PAVEMENT WITH VA SOUTH STATE PLANE GRID COORDINATE OF N:3611808.42, E:11459648.93 NAD 83(2011)(US SURVEY FEET);
THENCE, N41°12'48"W 11.88' TO A POINT ON EDGE OF PAVEMENT;
THENCE, N41°53'30"W 8.12' TO A POINT ON EDGE OF PAVEMENT;
THENCE, N49°02'57"E 132.81' TO A POINT;
THENCE, N40°57'03"W 30.00' TO A POINT;
THENCE, N49°02'57"E 20.00' TO A POINT ON PROPOSED LEASE PREMISE;
THENCE, WITH THE LEASE PREMISE S40°57'03"E 100.00' TO A POINT;
THENCE, LEAVING LEASE PREMISE S49°02'57"W 20.00' TO A POINT;
THENCE, N40°57'03"W 50.00' TO A POINT;
THENCE, S49°02"57"W 132.62' TO THE BEGINNING.

THE 20' WIDE INGRESS/EGRESS AND UTILITY/FIBER EASEMENT AREA BEING 4654 SQ. FT. AND LIES UPON T.M. NO. 46-A-50

LINE TABLE		
LINE	BEARING	LENGTH
L1	N41°12'48"W	11.88'
L2	N41°53'30"W	8.12'
L3	N49°02'57"E	132.81'
L4	N40°57'03"W	30.00'
L5	N49°02'57"E	20.00'
L6	S40°57'03"E	100.00'
L7	S49°02'57"W	20.00'
L8	N40°57'03"W	50.00'
L9	S49°02'57"W	132.62'

SEAL:

COMMONWEALTH OF VIRGINIA
MICHAEL RAY GOIN
Lic. No. 3143
6/30/23
LAND SURVEYOR

NOTES:
1. PROPERTY LINES SHOWN BASED ON RECORDED DEEDS AND PLATS. THIS IS NOT A BOUNDARY SURVEY.
2. PROJECT IS UNDER THE JURISDICTION OF PRINCE EDWARD COUNTY, VA.
3. SURVEY WAS PERFORMED ON 4/27/23 USING A SOKIA GRX3 RECEIVER WITH TOPCON NETWORK TO ESTABLISH CONTROL. DATA WAS COLLECTED WITH SOKIA ROBOT.

SURVEYOR OF RECORD:
DICKERSON SURVEYING LLC
500 Court Street P.O. Box 112
Appomattox, Virginia 24522
434-352-8560
Michael Ray Goin
Land Surveyor

ENGINEER OF RECORD:
BAMMAN CONSULTING, LLC
14489 ST ANDREWS LN
ASHLAND, VA 23005
PH: 703.328.8574

PROJECT:
STOCKTON LAKE

SITE INFORMATION:
4451 FIVE FORKS ROAD
PAMPLIN, VA 23958

SHEET NO:
2 OF 3

100' X 100' LEASE PREMISES DESCRIPTION

BEGINNING AT A POINT ON THE
SOUTHWESTERN CORNER OF THE
LEASE PREMISE AND THE MOST NORTHERN POINT OF
20' WIDE INGRESS/EGRESS AND UTILITY/FIBER EASEMENT
HAVING A COORDINATE OF
N:3611946.21, E:11459731.43 NAD 83(2011)(US SURVEY FOOT)
THENCE, N49°02'57"E 100.00' TO A POINT;
THENCE, S40°57'03"E 100.00' TO A POINT;
THENCE, S49°02'57"W 100.00' TO A POINT WITH EASEMENT;
THENCE WITH SAID VARIABLE EASEMENT
N40°57'03"W 100.00' TO POINT OF BEGINNING.

THE LEASE PREMISES ARE BEING 10,000 SQ. FT.
AND LIES UPON T.M. NO. 46-A-50

LINE TABLE		
LINE	BEARING	LENGTH
L10	S49°02'57"W	100.00'
L11	N40°57'03"W	100.00'
L12	N49°02'57"E	100.00'
L13	S40°57'03"E	100.00'

SEAL:

COMMONWEALTH OF VIRGINIA
MICHAEL RAY GOIN
Lic. No. 3143
6/30/23
LAND SURVEYOR

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